

Our Ref: CE18119 CGRC Water Main – Response to RFI dated 23 January 2024

Contact: Lloyd Moloney/Derek Arrowsmith

Att: Brendan Price The Dott Developments Pty Ltd Level 6 161 London Circuit Canberra ACT 2601

12 March 2024

#### Dear Brendan,

RE: Request for Information to aid the design process to extend and expand the provision of the Gundagai potable water supply to the DOTT development proposal at Lot 529B DP230601 and Lot 2 DP160191 – 37 Annie Pyers Drive, Gundagai

## Cootamundra-Gundagai Regional Council Request.

To support your proposed development CGRC have requested the following information to be provided in regard to water supply to DOTT for each stage (both individual and cumulative):

- a) Maximum flow rate required at any given time in litres per second.
- b) Maximum volume of water required over a 3, 6, 12, and 24 hour timeframe and a seven day timeframe. This information must include peak business time (Christmas Holiday Period). Such information is to be provided as litres per hour over each timeframe.
- c) Water demand and pressure requirements for Fire (Safety) Services. This information will need to be undertaken by an accredited Fire Service Engineer.

#### Moloney & Sons Engineering (MSE) Response

#### **Reference Documents**

Water System Planning for demand is completed in accordance with the Water Supply Code of Australia (WSCA). The relevant sections of the various relevant codes are:

- 1. WSCA, Part 1: Planning and Design, Section 2.2 Demands.
- 2. WSCA, Sydney Water Edition, Section 2 Water System Planning Guideline. This section directs designers to:
  - a. Sydney Water, Water System Planning Guideline, Section 2 Water Demand and Growth. (SW-WSDG)
- 3. WSCA, Regional New South Wales Edition, Section 2.3 Demands. (WSC-REG)
- 4. Moloney and Sons Hydraulic Design Report CE18119 Hydraulic Design Report\_001 Rev A, Attachment 3

The order of precedence for use of the design codes is 3, 2, 1.

#### **DOTT Development Area**

The subject properties are:

- Lot 2 DP 160191 with an area of 0.785ha
- Lot 529B DP 203601 with and area of 1.891ha
- Total Property Area 2.676ha the area used in this letter is rounded to 2.7ha



#### **Demand Calculation**

In accordance with WSC-REG Section 2.3.3.2 the system hydraulic design is for the total future demand based on land zoning for the required water supply. Water system design is not based on the estimated demand for staged development growth.

In accordance with WSC-REG Section 2.3.3.1 the demand rates for each type of development in the design area are to be obtained from the Water Agency. In the case of the DOTT the Water Agency is Goldenfields Water (GWCC) and Cootamundra Gundagai Regional Council (CGRC). GWCC do not provide demand rates for each local authority, the average day demand rate provided by CGRC for use in the MSE design report **Attachment 3** was 0.81kL/ET/day at a rate of 2.5 EP/ET the demand rate is 0.324L/EP/day. The subject properties in isolation were assigned, by CGRC to have a future residential population of 17ET (42.5 EP). The annual demand for the subject properties is 13,770L/dy. The maximum day and maximum hour (peak hour) are calculated in accordance with Table 2 in **Attachment 3**, as 27,540L/day and 5,738L/hr respectively. This equates to 1.59L/s for the full development area or 0.59L/s/ha.

The basis of demands adopted in **Attachment 3** was that all demand would be residential to give a conservative design.

To provide some validation of this demand rate reference was made to SW-WSDG section 3.3.2 Growth Demand Forecasting which requires the determination of the per capita consumption by demand category. Section 3.3.2 directs to Appendix C in SW-WSDG which provides demand categories but no demand rates. From Appendix C it is determined that the proposed land use will be either b) Commercial, or c) Special uses. The validation for the demand rate falls to WSCA Section 2.2.2, Table 2.1 Typical Peak Hour Demand Rates. There are no rates provided for Gundagai, it is therefore reasonable to assume that the Gundagai demand will be similar to either Canberra (without irrigation) or Sydney; 0.6L/s/ha and 0.9L/s/ha respectively; the average of these rates 0.75L/s/ha is adopted for the typical commercial peak hour demand rate. The land area of the subject properties is ~2.7ha, the total peak hour commercial demand is 2.025L/s.

The commercial demand rate, to be adopted, is 0.75L/s compared to the previously adopted residential demand rate of 0.59L/s.

#### **Design Usage**

Figure 2 in the MSE design report Attachment 3 provided a typical diurnal demand curve based on 30 minute usage periods for the entirety of the DOTT area subject to water supply. The development area is a small portion of this water supply area. The peak hour usage data for the development area in isolation, as residential (0.59L/s) and commercial (0.75L/s), are summarised in *Table 1*. In comparison with the usage data for the entire DOTT area from Attachment 3.

Period / Units	Calculation	Residential	Commercial	Trunk Main Design Report
Peak Hour L/s	None	1.59	2.025	10.875
Peak Hour L/h	Peak Hour L/s x 3600	5738	7290	39,150
Peak Day Demand L/day	Peak Hour L/h x 24	27,540	34,992	187,920
Consumption day L/day	Peak Day / 2	13,770	17,496	93,960
Consumption hour L/h	Consumption day / 24	573.8	729.0	3915.0
Consumption ½ hour L/30 min	Consumption hour / 2	286.9	364.5	1957.5

Table 1: Usage Calculations

Adjusting the usage of development area from the originally assumed basis of residential use to commercial use there is altered diurnal usage and demand patterns. The updated 30 minute diurnal usage pattern shows that the consumption occurs during differing time periods. As a result of the altered diurnal pattern there is a nett 17 % decrease in overall demand from the development site. The Diurnal curve for the development site based on residential and commercial along with the total daily demand curves are included in **Attachment 1**.



The results from the altered development site diurnal patterns have been applied to those previously reported Figure 2 in the MSE design report **Attachment 3** adjusted to show either the demand in L/30 min or cumulative volume as opposed to the usage factors. The Diurnal and cumulative supply curves are included in **Attachment 2**.

## **Response to Points Raised**

Item a) Maximum flow rate required at any given time in litres per second.

The peak hour flow rate for the DOTT Development Area as a commercial development is 2.025L/s.

Item b) Maximum volume of water required over a 3, 6, 12, and 24 hour timeframe and a seven day timeframe. This information must include peak business time (Christmas Holiday Period). Such information is to be provided as litres per hour over each timeframe.

The diurnal curves provided demonstrate the peak hour demand and usage over a 24 hour period, in ½ hourly increments, the peak hour takes account of the increased usage of holiday periods.

Item c) Water demand and pressure requirements for Fire (Safety) Services. This information will need to be undertaken by an accredited Fire Service Engineer.

WSC-REG requires no design allowance for fire flows unless the water agency specifies and allowance.

SW-WSDG advises there is no requirement placed on Sydney Water for provision of water for fire fighting.

WSCA Table 2.1 notes no requirement for fire demand allowance in either Sydney or Canberra regional areas.

MSE design report CE18119 Hydraulic Design Report\_001 Rev A, **Attachment C**, included a 15L/s allowance for hydrant demand in the pumping calculations. The pumped supply is based on delivery usage at 2.175L/s plus a hydrant demand of 15L/s. The total pumped supply for the entire DOTT area is based on 18L/s. The water demand and fire pressure supply have been provided in excess of the requirements for the New South Wales area outlined under the Water Supply Code of Australia. This does not void the site/facility specific necessary building code regulation requirements for wet & dry fire, which will be required by the applicant upon Building Approval application.

NOTE: All data and design calculations are provided for the full proposed development area as required by the Water Supply Code of Australia.



#### Conclusion

The development site when fully developed as a commercial site will cause an alteration of the diurnal demand patterns previously reported. The alteration to diurnal demand patterns is not detrimental to the currently designed water supply for the DOTT area, the alteration would not support reduction in pipe size or pumping capacity.

The development site when fully developed as a commercial site will reduce the cumulative volume of water supply required from the cumulative volumes upon which the current trunk water main design is based. The alteration to the cumulative supply volumes is not detrimental to the currently designed water supply for the DOTT area, the alteration would not support reduction in pipe size or pumping capacity.

Yours sincerely

Lloyd Moloney
Managing Director
for & on behalf of

MOLONEY & SONS™

Engineering

Derek Arrowsmith

Principal Engineer for & on behalf of

MOLONEY & SONS™

Engineering

Encl.

Cc via email:

Attachment: 1 – Development site diurnal and demand curves

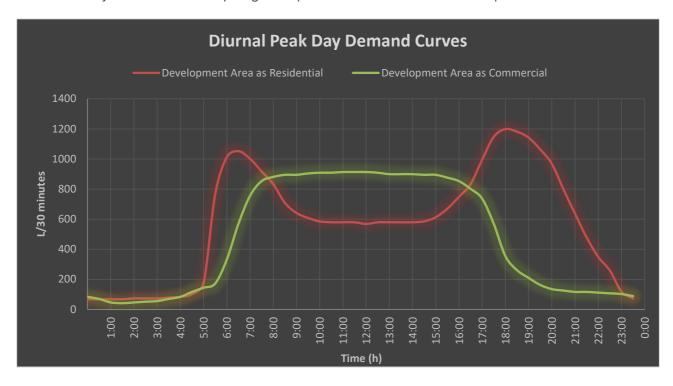
2 – MSE full report and adjusted diurnal and demand curves

3 – MSE Hydraulic Design Report CE18119 Hydraulic Design Report\_001 Rev A

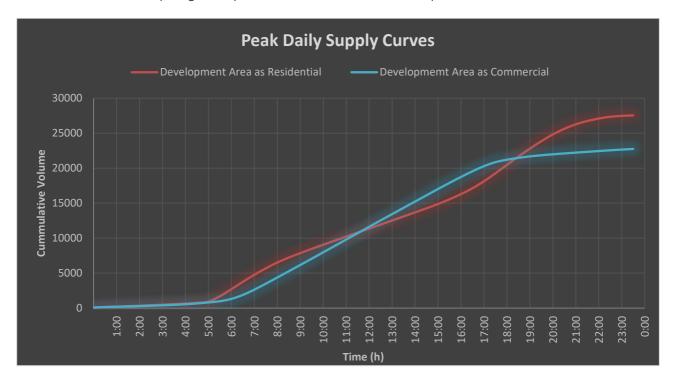


#### **Attachment 1**

Diurnal Peak Day Demand Curves comparing development site as residential with development site as commercial.



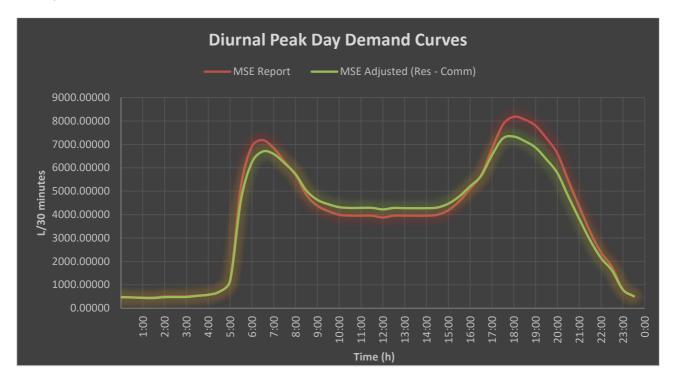
Total Demand Curves comparing development site as residential with development site as commercial.



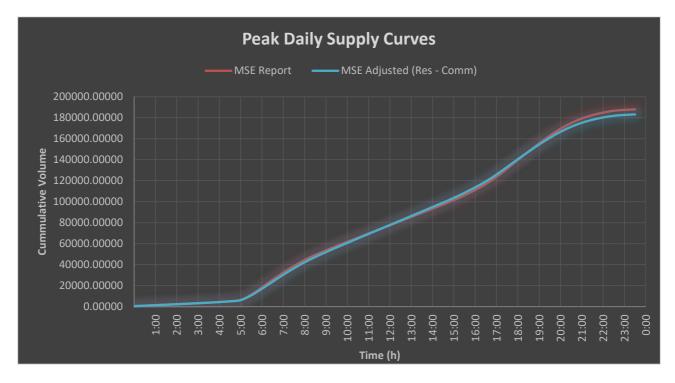


#### **Attachment 2**

Diurnal Curves comparing the full scope of the MSE report with the development site as residential and with development site as commercial.



Total Demand Curves comparing the full scope of the MSE report with the development site as residential and with development site as commercial.





## **Attachment 3**

MSE design report CE18119 Hydraulic Design Report\_001 Rev A





# COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL

Trunk Water Main – Springflat Drive to Coolac via The Dog on the Tuckerbox

HYDRAULIC DESIGN REPORT

22 May 2020

CE18119 Hydraulic Design Report\_001 Rev A

Contract No. CE19099



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2020

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## 1. INTRODUCTION

## 1.1. PROJECT COMMISSION

Moloney & Sons Engineering has been engaged by Cootamundra Gundagai Regional Council (CGRC) to provide detailed design for the construction of a trunk watermain from Gundagai to The Dog on the Tuckerbox (DoTB) and concept design for a trunk watermain from DoTB to the proposed Coolac Service Station and wider community.

#### 1.1.1. LIMITATIONS

This report is limited to providing a Hydraulic design that will provide suitable water supply to both the DoTB area and the Coolac community.

## 1.2. Project Overview

#### 1.2.1. BACKGROUND

CGRC provides water supply to the townships of Cootamundra and Gundagai. At present part of the DoTB area is supplied with partially treated water, pumped from Murrumbidgee River and stored in a concrete reservoir on the eastern side of the Hume Highway. The Coles Express, Shell Service station and the restaurants nearby are supplied with potable water from Gundagai town supply system by pumping water from Gundagai water system at the existing DN100 watermain located along Springflat Drive, Gundagai. This pump station and pipe work is privately owned and operated by the owners of premises.

CGRC has determined that population growth potential in the DoTB and Coolac areas will require potable water supply either in the short term (DoTB) or the long term (Coolac). This project has been established to confirm that the design of trunk watermain from Gundagai to DoTB will be capable of being extended to Coolac in the future.



## 1.2.2. LOCALITY

Figure 1 following provides a location context for the project.

Stage 1 Gundagi to DoTB Dog on the Tucker Proposed Hume Highway Underbore



Figure 1 Site Locality (Source: Google 2020)



## 1.3. DESIGN STANDARDS AND REQUIREMENTS

#### 1.3.1. DESIGN STANDARDS

The design standards and order of precedence for the project are as follows:

- WSA 03-2011-3.1 Water Supply Code of Australia (Sydney Water Edition);
- Goldenfields Water Standards; and
- Cootamundra Gundagai Regional Council Requirements.

## 1.3.2. DESIGN REQUIREMENTS

Following is a summary of the design requirements that have specific relevance to the project.

- Water supply is provided from Springflat Drive, Gundagai to the DoTB general area sufficient to service the current and potential future population (refer CGRC Population and Water Demand Estimation included in **Appendix A**).
- Ensure there is hydraulic capability and capacity in the main to provide future service and supply to the Coolac population area (refer CGRC Population and Water Demand Estimation included in **Appendix A**).
- The concept design alignments for stages 1 and 2 are included in **Appendix B**.

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## 2. HYDRAULIC DESIGN

#### 2.1. DESIGN ASSUMPTIONS

The proposed infrastructure demand has been based on CGRC's "Population and Water Demand Estimation" Report dated 15/04/20. Refer figure extract below.

Table 1 Equivalent Tenement Estimation (CGRC Population & Water Demand Estimation EXTRACT)

Area	ET
DoTB Tourist Zone	96
Outer area at the DoTB	20
En-route Coolac	15
Coolac Village	250
Total	381

Therefore, adopting a 116 ET, with 2.5 EP/ET for the Gundagai to DoTB supply and 265 ET, with 2.5 EP/ET for the DoTB to Coolac supply.

For the purposes of this study, the WSA demand criteria's have been adopted in order to achieve a preferred economical option. As it Moloney & Son's understanding adopting the CGRC Demands would result in excessive Reservoir Storage sizing, considering the current and planned Rural Zoning of the region therein.

Please refer below table for adopted parameters and demand.

Table 2 Water Supply Demand Flow Criteria

Parameter	CGRC Design Criteria (kL/ET/d)	WSA Design Criteria (kL/ET/d)	Adopted Criteria (kL/ET/d)
Average Day Demand	0.81	-	0.81
Max Day Demand	4	2*AD¹	2*AD
Max Hour Demand	7	5*AH (on Max Day)¹	5*AH (on Max Day)

<sup>&</sup>lt;sup>1</sup>In accordance with WSA Part 1 for populations below 2,000.

ABN: 39 133 970 689



Table 3 Estimated Average Demands

	Average Daily Consumption (L/d)	Max Day (2*AD) (L/d)	Max Hour (5*AH) (L/h)
DoTB Supply	93,960	187,920	39,150
Coolac Supply	214,650	429,300	89,438

#### 2.2. STORAGE DESIGN

For the purposes of this analysis and in order to establish an anticipated daily diurnal demand pattern for the subject infrastructure, the below typical demand pattern has been adopted and represented below.

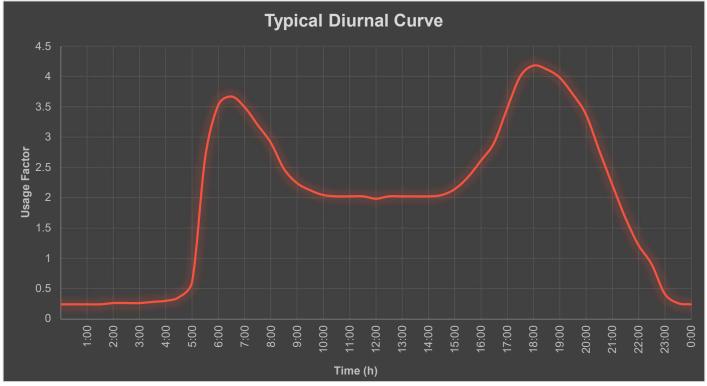


Figure 2 Typical MD Diurnal Demand Pattern

However, as the Diurnal Pattern is based upon the average 30-minute consumption. This curve/demand pattern has been corrected to account for a total daily consumption.

Refer APPENDIX C for the corrected interpreted table, converting the original factors in establishing an 8hr & 24hr pumping rate. The initial factors were required to be corrected as they did not account for the full use of the full 24hr cycle.



## 2.2.1. DOTB ESTIMATED STORAGE

Derived from the above diurnal pattern, the below Figure 3 illustrates the expected 8hr & 24hr pumping rates with the associated daily consumption volumes.

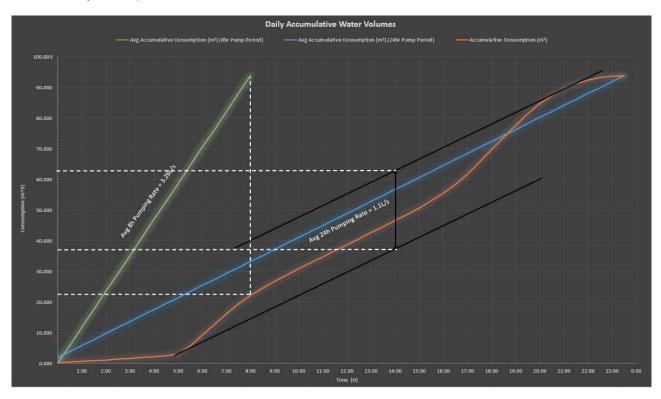


Figure 3 DoTB Daily Accumulative Consumption

Service Storage for pumping between 0000 hrs & 0800hrs:

$$V_{eq} = 93.96 - 22.5 = 71.46 \text{m}^3$$

Service Storage for pumping over 2400 hrs:

$$V_{eq} = 63 - 37 = 26 \text{m}^3$$

The required storage requirements are estimated from the  $\sum V_{eq}$  (equalising storage) +  $V_f$  (fire service) +  $V_{res}$  (reserve storage).

Where:

V<sub>res</sub> = 1/3 of the Peak/Max Day Demand as per WSA Part 1 Section 2.7

V<sub>eq</sub> = Max Day Demand

 $V_f = 15L/s$  from one hydrant for a 2-hr period, assumed under operation for purposes of this study



## 2.2.2. COOLAC ESTIMATED STORAGE

Please refer below expected 8hr & 24hr pumping rates with the associated daily consumption volumes.

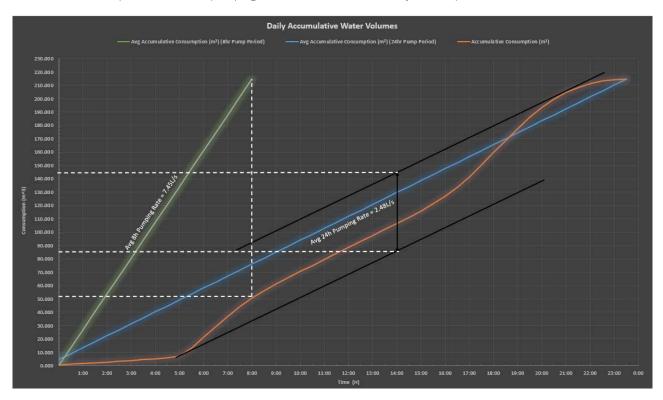


Figure 4 Coolac Daily Accumulative Consumption

Service Storage for pumping between 0000 hrs & 0800hrs:

 $V_{eq} = 214.650 - 52 = 162.65 \text{m}^3$ 

Service Storage for pumping over 2400 hrs:

 $V_{eq} = 145 - 85 = 60 \text{m}^3$ 

The required storage requirements are estimated from the  $\sum V_{eq}$  (equalising storage) +  $V_f$  (fire service) +  $V_{res}$  (reserve storage).

Where:

 $V_{res} = 1/3$  of the Peak/Max Day Demand as per WSA Part 1 Section 2.7

 $V_{eq}$  = Max Day Demand

 $V_f = 15L/s$  from one hydrant for a 2-hr period, assumed under operation for purposes of this study

Therefore, anticipated Service Storage Required is:



Table 4 Estimated Service Storage Requirements

Parameter	$V_{res}$	V <sub>f</sub>	$V_{\sf eq}$	V <sub>eq</sub>	Vreq'd Vol			
Unit	1/3*MD	15L/s for 2hrs	V <sub>8</sub>	V <sub>24</sub>	$V_{8hrs}$	V <sub>24 hrs</sub>		
	(m³)	(m³)	(m³)	(m³)	(m³)	(m³)		
DoTB Res.	62.640	108.000	71.460	26.000	242.100	196.640		
Coolac Res.	143.100	108.000	162.650	60.000	413.750	311.100		



## 2.3. Pumping Requirements

## 2.3.1. Springflat Dr to Dotb

The adopted required flow of the pump will be considered under a Maximum Daily Demand (L/s) condition plus an emergency Hydrant under use.

Therefore, as previously shown MD is given for L/d, whereas the pump will require the **2.175 L/s MD + 15 L/s** for a hydrant demand, therefore requiring a total flow of **17.175** L/s or ~18L/s.

The Pump head will be required to be:

$$h_p = \Delta z + \frac{fLV^2}{2gD}$$

Table 5 Springflat Dr Pump System Head Curve

	Pump System Head Curve												
Length (m)	Flow (m³/s)	ks	ø (mm)	A (m²)	V (m/s)	Re	ks/3.7D	5.74/Re^0.9	log(ks/3.7D+5.74/Re^0 .9)^2	f	ΔZ (m AHD)	hL=FLV2/2gD	hp = Δz + hL
3800	0.0172	0.0015	200	0.0314	0.547	1.09E+05	2.027E-06	1.674E-04	14.2201	0.017581	50.9	5.093	56.0

## Assumptions:

- Considering the lowest (Springflat Dr @ 249.1m AHD) & highest-pressure (STG 1 CH 2750 @ 300m AHD) location conditions.
- Length is total length from the Springflat Dr Pump to lowest pressure node @ DoTB Service Station.
- Flow is under the Max Daily Demand plus one (1) Hydrant rate in use

Therefore, the minimum required head at Springflat Dr will be **56.0 m.** 



## 2.3.2. Dotb (Annie Pyers Dr) to Coolac Service Reservoir

The adopted required flow of the pump will be considered under a Maximum Daily Demand (L/s) condition plus an emergency Hydrant under use.

Therefore, as previously shown MD is given for L/d, whereas the pump will require the **4.969 L/s MD + 15 L/s** for a hydrant demand, therefore requiring a total flow of **19.969** L/s or ~20L/s.

The Pump head will be required to be:

$$h_p = \Delta z + \frac{fLV^2}{2gD}$$

Table 6 Annie Pyers Dr Pump System Head Curve

	Pump System Head Curve												
Length (m)	Flow (m³/s)	ks	ø (mm)	A (m²)	V (m/s)	Re	ks/3.7D	5.74/Re^0.9	log(ks/3.7D+5.74/Re^0. 9)^2	f	ΔZ (m AHD)	hL=FLV2/2gD	hp = Δz + hL
11000	0.0200	0.0015	200	0.0314	0.636	1.27E+05	2.027E-06	1.462E-04	14.6621	0.017051	40.0	19.330	59.3

## Assumptions:

- Considering the lowest (Annie Pyers Dr @ 250m AHD) & highest-pressure (STG 2 CH 950 @ 290m AHD) location conditions.
- Length is total length from the Annie Pyers Dr Pump to lowest pressure node @ Coolac Service Reservoir.
- Flow is under the Max Daily Demand plus one (1) Hydrant rate in use

Therefore, the minimum required head at Annie Pyers Dr will be **60.0 m.** 



#### 2.4. Trunk Main Design

The proposed Trunk Main delivery line has been designed to cater for the Max Day Demand, which is to be pumped in the first 0800hrs, in accordance with WSA Guidelines and "Typical Capacities of Water Supply Systems", Water Resources Engineering (Chin, 2013).

In order to reduce pressure heads and pump loads, it is proposed that Stage 1 will consist of a ø200mm PE Trunk Main extension from the existing dead-end water supply connection at Springflat Dr through to the vicinity of Five Mile Creek Rd & Annie Pyers Dr intersection at the DoTB. Where it is proposed Stage 2 will later connect and continue to the proposed Coolac Service Reservoir and village community.

It is further proposed that a Ø100 PE reticulation line will service the DoTB Community and businesses, in accordance with above-mentioned and adopted EP demands.

#### 2.4.1. Pressure limits

The networks design pressures have been adopted in accordance Water Supply Code of Australia Part 1 Section 2.4, please refer below for adopted pressure limits:

Table 7 Service Pressure Limit (WSA Part 1 EXTRACT)

#### SERVICE PRESSURE (SP) LIMITS

SP Limit  Maximum	Application						
	Domestic	Industrial / Commercial					
	Licence or Water Agency requirement	Licence or Water Agency requirement					
Desirable Maximum	800 kPa (80 m)	800 kPa (80 m)					
Minimum (Note 1)	Licence requirement e.g. in the range 6-15 m	Licence requirement e.g. in the range 6-15 m					
Desirable Minimum	200 kPa (20 m)	250 kPa (25 m)					

Therefore, the adopted design Abs. Max Pressure is  $80m \times 9.79 = 783.2$  KPa and Min Pressure is  $20m \times 9.79 = 195.8$  KPa, as per STD SI unit conversion rates.

Table 8 DoTB Available Pressure Limits

Locality	Conditions	SP Limit	Domestic	Industrial/ Commercial	ΔZ (m AHD)	h <sub>L</sub> = fLV2/2gD	Available Pressure
DoTB Lowest Pressure Point	MD+HYD	Des Min	200 kPa (20m)	250kPa (25m)	300.0m – 257.0m = 43.0m	5.093	371 kPa (37.9m)

#### Assumptions:

- Considering the lowest (DoTB Service Station @ 257m AHD) & highest-pressure (STG 1 CH 2750 @ 300m AHD) location conditions.
- Length is total length from the Springflat Dr Pump to lowest pressure node @ DoTB Service Station.
- Flow is under the Max Daily Demand plus one (1) Hydrant rate in use



Therefore, Table 8 above demonstrates adequate available pressures at the DoTB to sufficiently service the community without additional elevated storage, provided the Gundagai network can meet the specified supply demands.

Table 9 Coolac Available Pressure Limits

Locality	Conditions	SP Limit	Domestic	Industrial/ Commercial	ΔZ (m AHD)	h <sub>L</sub> = fLV2/2gD	Available Pressure
Coolac Lowest Pressure Point	MD+HYD	Des Min	200 kPa (20m)	250 kPa (25m)	261.0m – 244.0m = 17.0m	1.003	156 kPa (16m)

## Assumptions:

- Considering the lowest (*Coolac Service Station @ 244m AHD*) & highest-pressure (STG 2 *Coolac Service Reservoir @ 261m AHD*) location conditions.
- Length is total length from the future Coolac Service Reservoir to lowest pressure node @ Coolac Service Station.
- Flow is under the Max Daily Demand plus one (1) Hydrant rate in use

As shown in the above Table 9, there is **insufficient** pressure head available at the proposed Coolac Service Station (lowest pressure node) if the normal operating level where to be @ 261m.

Table 10 Head Loss

	Springflat Dr to DoTB Service Station (Low Pressure Elevation)						
	Length (m)	Flow (m³/s)	ks	ø (mm)	V (m/s)	f	h <sub>L</sub> = fLV2/2gD
MD+HYD Conditions	571	0.0200	0.0015	200	0.636	0.017	1.003

Thus, giving rise to a  $z_0$  of:

$$z_0 = \frac{p_{min}}{y} + z_{min} + h_L$$

$$\therefore z_0 = 20 + 244 + 1.003 = 265 \text{ m}$$

This consequentially suggests the future Service Reservoir must achieve a normal operating base level of 265m AHD as a minimum or CGRC have the alternative option of opting for a Pumped pressurised supply to Coolac from the Service Reservoir.



## 3. SUMMARY

The purpose of this report is to provide a hydraulic analysis for the provision of a trunk watermain to service both the Dog on the Tuckerbox and the Coolac local community areas. The primary potable water supply will be provided from the existing Gundagai water network at Springflat Dr. The analysis indicates that a 200mm diameter PE trunk main will provide for all future development when supported by local supply networks.

The analysis makes the assumption that there is sufficient supply capacity in the Gundagai water network to support supply via Dog on the Tuckerbox to Coolac.

Construction of the watermain is proposed to be over more than one stage with the first stage being the supply from Gundagai to the Dog on the Tuckerbox. The hydraulic analysis for this stage shows that once the water supply has been pumped past the highest point of the alignment there would be no requirement for additional service supply storage. Future consideration could be given to providing an emergency standby supply reservoir on the western side of the Hume Highway, but this would not be able to provide potable water without turnover of water usage.

The following stage will extend the trunk main from Dog on the Tuckerbox to Coolac. The hydraulic analysis for this stage shows that a pumped network is required, however, the pumping should not exceed the maximum allowable pressure for service supply to the 15 properties that the main will be required to supply. The necessary pump will be sufficient to provide supply to a storage reservoir at Coolac, currently located on the highest point close to the Coolac Service Station to the west of the Hume Highway. This reservoir, unless elevated, will not provide sufficient pressure to service the community without supply pumping.



## 4. RECOMMENDATION

Following are the recommendations resulting from this analysis report:

- 1. The trunk main should be 200mm diameter PE.
- 2. The trunk main should be extended from Gundagai directly to Dog on the Tuckerbox without the use of a storage reservoir.
- 3. The water supply at Springflat Drive will require pumping to maintain supply pressure.
- 4. Water supply to the Dog on the Tuckerbox community will be via a 100mm diameter PE distribution network tapped to the trunk main.
- 5. The trunk main from Dog on the Tuckerbox to Coolac will require pumping at Annie Pyers Drive to maintain supply pressure to Coolac.
- 6. Water supply for Coolac will require a Service Reservoir.
- 7. The water storage reservoir at Coolac will require either elevation or pumping to the distribution network.



## APPENDIX A - CGRC POPULATION AND WATER DEMAND ESTIMATION





# DOG ON THE TUCKER BOX & COOLAC WATER SUPPLY

Population and Water Demand Estimation

Water Supply Extension from Gundagai to Dog on Tucker the Box and Coolac

## **Document Control**

Version	Author	Reviewer	Comments	Date
Draft				18/03/2020
Revision 1	1 Revised u		Revised using Planning	15/04/2020
			Proposal No.4	

# Acronyms

AC Asbestos Cement

DICL Ductile Iron

DPIE Department of Planning, Infrastructure and Environment

EP Equivalent Population ET Equivalent Tenement

GL Giga litre

GWCC Golden Water County Council

LWU Local Water Utility

ML Million Litres

MPVC Modified Poly vinyl Chloride
OPVC Oriented Poly vinyl Chloride

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## 1 Introduction

## 1.1 Background

Cootamundra Gundagai Regional Council provides water supply to the townships of Cootamundra and Gundagai and part of the Dog on the Tucker Box (DOTB). At present, part of the DOTB area is supplied with partially treated water, pumped from Murrumbidgee River and stored in a concrete reservoir and distributed. The Coles Express, Shell Service station and the restaurants nearby are supplied with potable water from Gundagai town supply system by pumping water from Gundagai water system by accessing the exiting DN100 watermain located along Springflat Drive, Gundagai. This pump station and pipe work is privately owned and operated by the owners of premises.



Figure 1 - Dead End Water Main Springflat Drive

This document deals with the potential population projection which will be served by the proposed scheme and the water demand.

## 1.2 Project objectives

The objectives of this projects includes;

- Estimate the population and water demands for the proposed water supply extension to Dog on Tucker Box area and extension to Coolac Village.
- Concept design and detailed design of transmission pipe lines, storage reservoirs and reticulation system with the all associated hydraulic calculations to DOTB
- Concept design and asset sizing including pipe lines and storage reservoirs including hydraulic calculations for the future extension of the water supply system to Coolac village
- Develop technical specification and tender documents to invite tenders for water supply to DOTB.

## 2 Existing Water Supply Scheme

## 2.1 Gundagai Town Water Supply Scheme

Gundagai Town has a water supply system serving approximately 2500 EP both domestic, commercial and institutions. The raw water source for the Gundagai Water Supply System is the Murrumbidgee River and the extraction point is downstream of Burrinjuck Dam and the junction with Tumut River. The Gundagai Local Water Utility (GLWU) has a high security town water supply licence for 1,250 ML/yr.

The existing water treatment plant is a conventional plant consisting of with chemical dosing (alum) / flash missing, sedimentation, sand filtration, disinfection fluoridation and pH correction unit processes. The Water Treatment Plant has a rated output of 0.25 ML per hour and capable of producing 5 MLD.

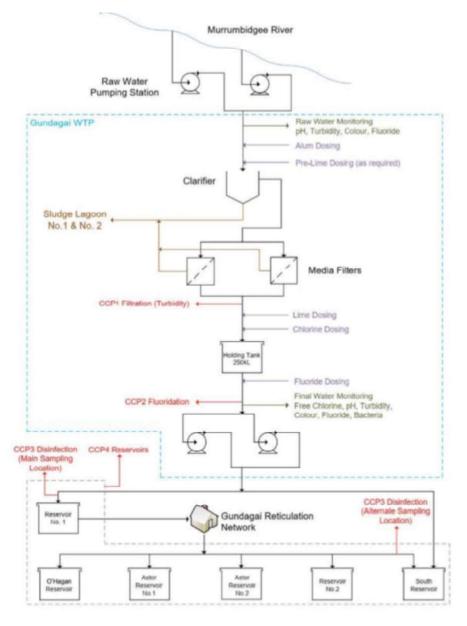
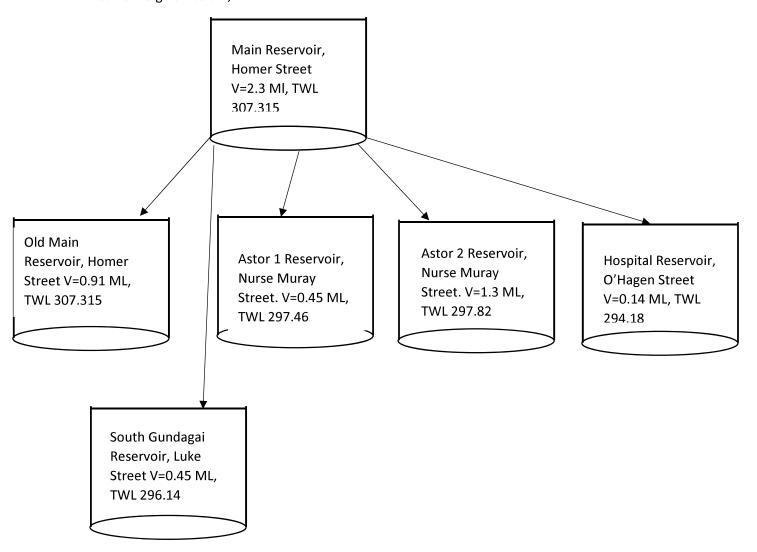


Figure 2 - Gundagai Water Supply system

## 2.1 Gundagai Town Water Supply Reservoir Capacities

Gundagai water supply system reticulation reservoirs has a combined capacity of 5.5 ML and the details are given below;



## 2.2 DOTB Water Supply Scheme

There are two separate water supply system at the Dog on Tucker Box and surrounding businesses.

A private water supply scheme which supplies to the Service station, cafes and restaurants by pumping water from the Gundagai Town Water Supply system. This water supply systems taps the dead end DN100 pipe along Springflat Drive, Gundagai and pumps it through a rising main to supply the service station, restaurants and cafes.

A water supply systems exists for the Dog on Tucker Box tourist premises and souvenir shop. Raw water is pumped from the Murrumbidgee River and stored in concrete reservoir and delivers through a DN90 pipe laid across Hume Highway to the premises with some partial treatment and the details of the treatment is not fully known.

# 3 Planning and Land Zoning

## 3.1 Current Planning and Land Zoning

## 3.1.1 DOTB and surrounds

Gundagai Planning identifies the area where the Dog on Tucker Box and other services Shell Service Station and restaurants a as SP3 Tourist with minimum lot size of 1500 Sq.m and area perimeter of the Dog on the Tucker Box as RU1 Primary Production with minimum Lot size AB2 40 Ha.



Figure 3- Land Zoning at Dog on the Tucker Box

14	RU1 Primary Production
	SP3 - Tourist

## 3.1.2 Coolac

# Coolac is zoned mostly SP3 and RU1



Figure 4- Zoning Coolac Village



## 3.1 Proposed Planning and Land Zoning

Cootamundra Gundagai Regional Council's Planning Policy No. 4, proposes to change the zoning around Coolac village. Details of this proposal and the potential impact on future Lot numbers (ET) are given in Appendix B.

# 3 Population and Water Demands

## 3.1 Cootamundra Gundagai LGA

The population projections details are provided in Appendix A of this documents.

Department of Planning NSW figures shows the following population for Cootamundra Gundagai Regional Council

Table 1 - Population Forecast Cootamundra Gundagai Regional Council

Year	CGRC Population		
21 (2036)2011	11,300		
2016	11,250		
2021	11,100		
2026	10,800		
2031	10500,		
2036	10,100		

During this period the average household size reduces from 2.33 (2011) to 2.21 (2031). The above data represents the entire Council and the town centres tend to have denser population. Therefore for Water supply demand estimation a house hold size of 2.5 is used.

## 3.2 Dog on the Tucker Box and Coolac

The proposed area comes under zoning SP3 – Tourist and RU1 Primary Production. On the basis of proposed lot sizes the following potential ET is estimated. The details are given in Appendix A.;

Table 2 - Equivalent Tenement Estimation

Area	ET	
Dog on the Tucker Box	96	
Outer area at the DOTB	20	
En-route Coolac	15	
Coolac Village	250	
Total	381 (say 400)	

Assumed population of 2.5 EP per ET

Table 3- Equivalent Population Estimate

Area	ET
Dog on the Tucker Box Tourist Zone	96
Outer area at the DOTB	20
En- route Coolac	15
Coolac Village	250
Total	381 (say 400)

Future rezoning could impact on the ET and water demand which need to be funded by the developer charges. The above ET calculation takes into account of Council's Planning Proposal No. 4 which identified the following (refer Appendix B for detail)

- Rezone the SP3 Tourism to RU5 Village 19ha in area with a Minimum Lot Size (MLS) of 2,000m<sup>2</sup> to the proposed RU5 area, Potential lot yield of 95 lots of 2,000m<sup>2</sup>
- Rezone some RU1 Primary Production close to the village to RU4 Primary Production Small Lots as shown below in green approximately 190ha in area with a Minimum Lot Size (MLS) of 2ha to the proposed RU4 area, Potential lot yield of 95 lots of 2ha
- Rezone some RU1 Primary Production close to the village to IN1 General Industrial as shown below in purple approximately 16.5ha in area; no minimum lot size for IN1 area. 60 lots allowed for ET and water demand calculation.

Total ET for Coolac Village used in this report is e water demand estimate is 250.

#### 3.3 Water Demand

Based on the above population predication the proposed water supply scheme will serve for 1000 EP with 400 ET in the future. This could allow for potential growth in the foreseeable future.

## 3.3.1 Demand Estimation using Golden field County Council criteria

The design average daily demand (ADD) and Maximum Daily Demand (MDD) for the proposed water supply extension is given in the Table below. The design criteria are taken from the GWCC Drinking Water Management System. ADD is given as 294 kL/y/ET which is equal to 0.81 kL/d/ET in the Table.

It is possible there will be rural property connections in the future and it is assumed that 10 rural property connections will be made along the route of the pipe line.

Table 4 - Water Demand Peak Flow Estimation

Parameter	Design Criteria(kL/ d/ET)	Total Demand (kL/d)	Potential Rural Demand along the Route of the proposed pipe line		
			Design Criteria for Rural Connection(kL/d)	Rural Demand (kL/d)	Total Demand (KL/d)
Average Day Demand	0.81	324	5	50	374
Maximum Daily Demand	4	1,600	25	250	1,850
Maximum Hour Demand	7	2,800 (32.4 L/s)	45	450	3,250 (37.6 L/s)

A typical residential diurnal patter is applied to determine the maximum hour requirements. The diurnal patter has a peaking factor of 1.8 which occurs around 6 pm.

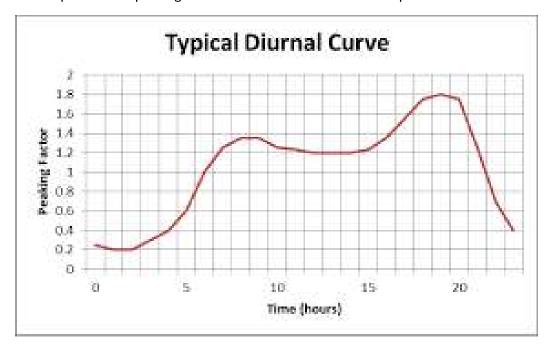


Figure 5 - Diurnal Water Us Pattern

#### 3.3.1 Demand Estimation using Gundagai Town Centre Consumption

#### **Gundagai Consumption**

Gundagai town water consumption patter indicate daily consumption of 400 L/ET with a peak daily consumption of 2,960 L. A figure of 3,000 L is used.

Table 5 - Water Demand Estimation Gundagai

Parameter	Design Criteria(kL/d/ET)	Total Demand (kL/d)	Potential Rural Demand along the Route of the proposed pipe line		he Route of
			Design Criteria for Rural Connection(kL /d)	Rural Demand (kL/d)	Total Demand (KL/d)
Average Day Demand	0.40	160	5	50	210
Maximum Daily Demand	3	1,200	25	250	1450
Maximum Hour Demand	6	2,400 (27.8 L/s)	45	450	2,850 (33.L/s)

### 3.4 Preliminary Hydraulic Calculation

The following table provides velocities in various sizes of pipes at 33 L/s flow rate which would be peak hourly demand. Transmission pipe line can be designed for a lower flow rate of peak daily demand provided the supply zone storage reservoirs have adequate capacity which will enable to meet peak hourly demand. The following table provides velocities in various types of pipe line at 30 L/s and 35 L/s.

Table 6- Velocities and Pipe sizes

Pipe Type	size	Grade	Internal	Velocity (m/s)	Velocity (m/s)		
			Dia.(mm)	At 30 L/s	At 35 L/s		
OPVC	DN150	PN18	166.9	1.37	1.6		
	DN200		218.8	0.80	0.93		
mPVC	DN150		158.2	1.53	1.8		
	DN200		207.3	0.9	1.05		
DICL	DN150	PN35	167	1.37	16		
	DN200		222	0.77	0.9		

Based on the initial assessment it appears that DN150 pipe would be adequate to meet peak flows. However, detailed calculations must be done to limit the head loss and velocities within the acceptable range. DN200 would allow for future high growth.

#### 3.5 Treatment Plant Capacity Constraints

The capacity of the Gundagai Water Treatment Plant of 5 MLD comes into consideration. Based on the available records 1.3 ML/ day has been the average production. However a peak demand occurred in January 2014 with a record daily production of 4.1 ML/day which is just 14% below the capacity of Gundagai Water Treatment Plant.

Additionally, the plant has limited redundancy with only one clarifier and limited land availability for expansion.

# Appendix A – Population Data

Table 7 - Department of Planning Information

## COOTAMUNDRA GUNDAGA REGIONAL COUNCILI

TOTALS:	2011	2016	2021	2026	2031	2036
Total Population	11,300	11,250	11,100	10,800	10,500	10,100
Total Households	4,800	4,800	4,800	4,700	4,600	4,450
Average Household Size	2.33	2.30	2.27	2.24	2.23	2.21
Implied Dwellings	5,600	5,650	5,600	5,500	5,350	5,200
CHANGE:		2011-16	2016-21	2021-26	2026-31	2031-36
Total Population Change		-50	-200	-250	-350	-400
Average Annual Population Growth		-0.1%	-0.3%	-0.5%	-0.6%	-0.7%
Total Household Change		50	-50	-100	-100	-150
Average Annual Household Growth		0.2%	-0.1%	-0.4%	-0.5%	-0.6%
		2010				
AGE GROUPS:	2011	2016	2021	2026	2031	2036
0-4	650	650	650	650	600	550
5-9	800	650	700	650	650	600
10-14	750	750	650	650	650	650
15-19	700	650	650	550	550	550
20-24	500	550	500	450	400	350
25-29	500	550	550	500	450	400
30-34	500	550	550	550	500	450
35-39	600	500	550	550	550	500
40-44	650	600	550	600	600	550
45-49	800	650	600	550	600	600
50-54	800	800	650	600	550	600
55-59	800	800	750	650	600	550
60-64	850	750	800	750	650	600
65-69	700	800	750	750	750	650
70-74	650	650	750	700	700	700
75-79	400	550	550	650	600	650
80-84	350	350	450	450	550	500
85+	300	350	400	500	550	650
HOUSEHOLD TYPES:	2011	2016	2021	2026	2031	2036
Couple only	1,550	1,600	1,600	1,600	1,500	1,450
Couple with children	1,250	1,200	1,150	1,100	1,100	1,050
Single parent	450	450	400	400	400	400
Other family households	450 50	430 50	50	50	50	50
Other failing households	30	30	30	30	30	30

Multiple-family households	50	50	50	50	50	50
Total family households	3,300	3,300	3,250	3,150	3,050	2,950
Lone person	1,400	1,450	1,450	1,450	1,450	1,450
Group	100	100	100	50	50	50
Total non-family households	1,500	1,500	1,550	1,550	1,500	1,500
Total	4,800	4,800	4,800	4,700	4,600	4,450

#### Disclaimer

While every reasonable effort has been made to ensure that these projections are correct at the time of release, the State of New South Wales, its agents and employees, disclaim any and all liability to any person in respect of anything or the consequences of anything done or omitted to be done in reliance upon the whole or any part of these projections.

Table 8 - Land Details near DOTB zoning SP3

Land Parcel Title	Address	Lot Size(Sq.m)	Potential ET	Comments
Lot: 7 DP: 854192 11 Annie Pyers Drive		133,994	40	Commercial
	Gundagai			Coles express
Lot: 3 DP: 785083	31 Annie Pyers Drive, Gundagai	9,352	6	
Lot: 529B DP: 203601	37 Annie Pyers Drive	18,792	12	
Lot: 2 DP: 160191	37 Annie Pyers Drive	7,798	5	
Lot: 294 DP:	12 Five Mile Creek	7.993	5	
751421	Road			
Lot: 295 DP:	0 Five Mile Creek	38.648	14	
751421	Road			
Lot: 2 DP: 264315	0 Annie Pyers Drive	3,523	2	
Lot: 1 DP: 264315	34 Annie Pyers Drive	9,039	6	Caltex
Lot: 5 DP: 263387	0 Annie Pyers Drive	31,699		Caltex Oil
Lot: 1 DP: 1191344	50 Annie Pyers Driv	1,655	1	
Lot: 7 DP: 263387	0 Annie Pyers Drive	2.665	2	
Lot: 21 DP: 263387	0 Hume Highway	4.765	3	
	Total		96	

Table 9 - Land Details near DOTB zoning RU1 - Primary Production

These lots are not the minimum size 40Ha as specified and subdivided into smaller lots. It is assumed that these lot will not be subjected to further subdivision and 1 ET per lot is assumed.

Land Parcel Title	Address	Lot Size(Sq.m)	Potential ET	Comments
Lot: 8 DP: 1244273	0 Five Mile Creek Road.	80,556	1	
	Gundagai			
Lot: 94 DP: 1209439	9546 Hume Highway,	331,110	1	
	Gundagai			
Lot: 7 DP: 1244273	0 Five Mile Creek Road	72,230	1	
Lot: 6 DP: 1164630	108 Five Mile Creek Road	77,183	1	
Lot: 4 DP: 1164630	0 Five Mile Creek Road	77,936	1	
Lot: 6 DP: 854192	0 Five Mile Creek Road	32,995	1	
Lot: 2 DP: 785083	0 Gundagai Shire Parish	33,107	1	
Lot: 1 DP: 1164630	0 Hume Highway	79,226	1	
Lot: 2 DP: 1164630	79 Five Mile Creek Road	72,614	1	
Lot: 3 DP: 1164630	0 Hume Highway	71.380	1	
Lot: 101 DP: 751421	95 Five Mile Creek Road	164,593	1	
Lot: 183 DP: 751421	118 Five Mile Creek Road	170,822	1	
Lot: 577 DP: 751421	2 Five Mile Creek Road	246,709	1	
Lot: 11 DP: 263387	0 Hume Highway	155,717	1	
Lot: 10 DP: 263387	0 Hume Highway	196,736	1	
Lot: 9 DP: 263387	0 Hume Highway	157,459	1	
Lot: 8 DP: 263387		208,587	1	
Lot: 1 DP: 717708	9321 Hume Highway	77,991	1	
Lot: 2 DP: 717708	0 Hume Highway	205,714	1	
Lot: 7 DP: 264315	0 Hume Highway	95,121	1	
	Total		20	

Table 10 - Land Details near Colac zoning SP3

Land Parcel Title	d Parcel Title Address		Lot	Potential	Comments
			Size(Sq.m)	ET	
Lot 1DP119876	Coolac Road	SP3	15394	10	
Lot 7304	Hume Highway,	SP1	32,814	21	Special activity
	Coolac				

PLT: 830 DP:			36,069	25	
1178634			30,003	23	
1170054					
Lot: 1 DP: 1124311	Coolac Road,	RU1	10,126	2	AB2 40 Ha
2011 2 311 222 1022	Coolac		13,123	-	7.52 10114
Lot: 4 DP: 1125835	Coolac	RU1 and	1.843,536	5	Large lot section
		SP3			facing Coolac Rd.
					is zoned SP3
PLT: 830 DP:	Coolac Road,	SP3	36.069	24	10 201100 07 0
1178634	Coolac				
Lot: 21 DP:	Coolac Road	RU1 and	225,834	27	Large lot section
1049165		SP3			facing Coolac Rd.
					is zoned SP3
Lot: 3 DP: 1125835	427 Coolac Road,	SP3	8.073	5	
	Coolac				
Lot: 2 DP: 591526	427 Coolac Road	SP3	1312	1	
Lot: 237 DP:	431 Coolac Road,	SP3	1257	2	Coolac Hall
753599	Coolac				
PLT: 316 DP:	437 Coolac Road	SP3	39,160	26	
753599					
Lot: 127 DP:	17 School Road.	SP3	8086	7	
753599	Coolac				
Lot: 263 DP:	School Road,	SP3	4060	2	Church Coolac
753599	Coolac				
Lot: 262 DP:	453 Coolac Road,	SP3	1701	2	
665914	Coolac				
Lot: 1 DP: 409642	6 School Road.	SP3	20214	14	
	Coolac				
Lot: 22 DP:	477 Coolac Road,	SP3	1573	2	Coolac Motel
1187297	Coolac				
Lot: 21 DP:	479 Coolac Road,	SP3	3047	2	Coolac Hotel
1187297	Coolac				
Lot: B DP: 394628	Muttama Road	SP3	562	1	
Lot: A DP: 394628	Muttama Road	SP3	593	1	
Lot: E DP: 411988	Muttama Road	SP3	2097	1	
Lot: D DP: 411988	Muttama Road	SP3	1178	1	
Lot: 1 DP: 384612	495 Coolac Road,	SP3	1242	1	
	Coolac				
Lot: 2 DP: 813973	503 Coolac Road,	SP3	2648	1	
	Coolac				
Lot: 10 DP:	Coolac Road,	RU1	166,350	5	New Service
1242413	Coolac				statin
	Total			187	

Refer Appendix B for revised Lot numbers for Coolac village.

Table 12 - Along the route of the Proposed Pipeline

Land Parcel Title	Address	Lot Size(Sq.m)	Zoning	Potential ET	Comments
Lot10 DP748275	9086 Hume Highway	50,827	RU1	3	AB2 40 Ha
Mingay Rest Area				5	RMS Truck stopping station
Vehicle Inspection Station				2	
Residences			RU1	5	
	Total			15	

## Appendix B – Impact of Council's Planning Policy Proposal No.4

Council's Planning Proposal No. 4 will increase the potential lot numbers and water demand. The following is the extract from the Planning Proposal No. 4 (PP 4). It must be noted that, at this stage this is being a proposal and not yet approved.

However, if approval is granted this would be the ultimate ET numbers which will

### Part 2 – Explanations of Provisions

The proposed outcomes are to be achieved via the following methods:

#### 1A. COOLAC

- Rezone the SP3 Tourism to RU5 Village as shown below in red approximately 19ha in area;
  - Provide a Minimum Lot Size (MLS) of 2,000m<sup>2</sup> to the proposed RU5 area
  - Potential lot yield of 95 lots of 2,000m<sup>2</sup>
- Rezone some RU1 Primary Production close to the village to RU4 Primary Production Small Lots as shown below in green approximately 190ha in area;
  - Provide a Minimum Lot Size (MLS) of 2ha to the proposed RU4 area
  - Potential lot yield of 95 lots of 2ha
- Rezone some RU1 Primary Production close to the village to IN1 General Industrial as shown below in purple approximately 16.5ha in area;
  - no minimum lot size for IN1 area

## **Existing Zones (SP3 Tourist & RU1 Primary Production)**

## **Proposed Zoning**



Approximate location of proposed change

**Proposed LEP** 



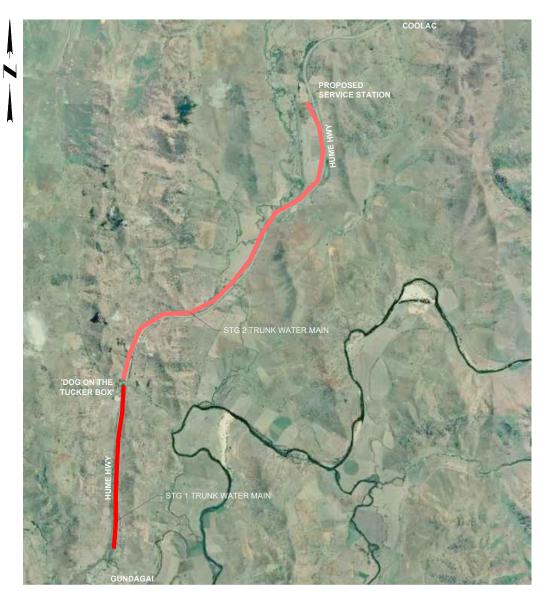
## APPENDIX B - CONCEPT DRAWINGS

# COOTAMUNDRA-GUNDAGAI REGIONAL COUNCIL

# **GUNDAGAI TO FIVE MILE TRUNK WATER MAIN**

# STAGE 1





LOCALITY PLAN

DETAIL SURVEY BY: ALLSPEC & PARTNERS - DATE SURVEYED 11/07/18 SURVEY: MGA94 ZONE 55 STANDARD DRAWINGS:

CGRC STANDARD DESIGN DRAWINGS, SYDNEY WATER STD DRAWINGS & SPECIFICATIONS, RMS GUIDELINES & INSTITUTE OF PUBLIC WORKS ENGINEERING AUSTRALIA (IPWEA)

#### PROPOSED TRUNK WATER MAIN - 2019

DESIGN FILE No: CE18119

DESIGN STANDARD: CGRC STD DRGS, AUSTROADS & RMS DESIGN GUIDELINES

#### **GENERAL**

- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONSULTANTS DRAWINGS AND SPECIFICATIONS
- BEFORE PROCEEDING WITH THE WORK ANY DISCREPANCIES IN THE CONTRACT DOCUMENTS SHALL BE REFERRED FOR DECISION TO THE SUPERINTENDENT
- DO NOT SCALE FROM DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL LOCATIONS OF SERVICES, ALL DIMENSIONS AND LEVELS PRIOR TO CONSTRUCTION.
- ALL MATERIALS/CONSTRUCTION & WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE CONTRACT SPECIFICATIONS AND THE LOCAL AUTHORITY'S STANDARD DRAWINGS AND BY-LAWS.
- THE CONTRACTOR IS RESPONSIBLE TO OBTAIN ALL RELEVANT APPROVALS PRIOR TO COMMENCEMENT OF WORKS
- UNDERGROUND SERVICE LOCATIONS SHOWN ON THIS PLAN HAVE BEEN DETERMINED BY FIELD SURVEY AND/OR OFFICE RECORDS, AND MAY NOT REPRESENT ALL SERVICES OR EXACT LOCATIONS. THE CONTRACTOR MUST ACCURATELY LOCATE AND DEPTH ALL SERVICES LIKELY TO
- BE ENCOUNTERED DURING CONSTRUCTION. PRIOR TO COMMENCING ANY EXCAVATION WORKS DISPERSIVE SOILS ARE NOT TO BE USED AS FILL/EMBANKMENT MATERIAL. ALL CONSTRUCTION TO BE DONE IN ACCORDANCE WITH CURRENT RMS SPECIFICATIONS AND TECHNICAL STANDARDS

#### WATER & WASTE WATER

- CONSTRUCTION AND MATERIALS SHALL BE UNDERTAKEN AND PROVIDED IN ACCORDANCE WITH
- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE WSAA SUPPLY CODE & DRAWINGS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ALL DISCREPANCIES SHALL BE REFERRED TO THE SUPERINTENDENT FOR DIRECTION BEFORE PROCEEDING WITH WORK.
- DIMENSIONS SHALL NOT BE SCALED FROM DRAWINGS.
- SET-OUT DIMENSIONS AND LEVELS, INCLUDING ANY SHOWN ON THE DRAWING ARE TO BE
- VERIFIED ON SITE BY THE CONTRACTOR BEFORE FABRICATION AND CONSTRUCTION.
  THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF PIPES AND PITS AGAINST FLOATATION DURING CONSTRUCTION.
  NO SUBSTITUTE MATERIAL SHALL BE USED WITHOUT APPROVAL OF THE SUPERINTENDENT.
- HEIGHT DATUM IS TO AHD AND COORDINATES ARE RELATIVE TO MGA94 (ZONE56). SURVEY EXTENTS ARE GENERALLY LIMITED TO THE PROPOSED WATER MAIN ALIGNMENT AND DO
- NOT NECESSARILY INCLUDE ALL EXISTING FEATURES WITHIN THE PROJECT AREA. UNDERGROUND SERVICE UTILITIES HAVE BEEN PLOTTED USING SERVICE AUTHORITY RECORDS
- AND MAY NOT REPRESENT THE ACTUAL SERVICE LOCATION. WHERE REQUIRED, THE PLOTTED LOCATION OF THESE SERVICES MAY HAVE BEEN ALTERED TO FIT ACTUAL SURVEYED SERVICES

- WHERE CONNECTING TO EXISTING PIPEWORK, THE LEVEL AND DIAMETER OF THE EXISTING PIPEWORK SHALL BE CONFIRMED BY THE CONTRACTOR PRIOR TO CONNECTION.
- CHANGES IN HORIZONTAL AND VERTICAL ALIGNMENT, OTHER THAN THROUGH FABRICATED BENDS, SHALL BE ACHIEVED BY DEFLECTION AT PIPE JOINTS WHERE SHOWN. MAXIMUM DEFLECTION SHALL BE AS PER MANUFACTURER'S SPECIFICATIONS.
- PROVIDE SUFFICIENT TRENCH DEPTH AND WIDTH TO ALLOW FOR DEFLECTION OF PIPES AT
- PRESSURE MAIN CLEARANCE FROM ALL EXISTING SERVICES SHALL BE IN ACCORDANCE WITH THE WSAA SUPPLY CODE. IF CLEARANCE BETWEEN SERVICES IS LESS THAN THAT SPECIFIED, REFER TO THE SUPERINTENDENT FOR DIRECTION BEFORE PROCEEDING WITH WORK.
- MINIMUM PRESSURE MAIN COVER FROM FINISHED GROUND LEVEL TO THE OBVERT OF THE MAIN SHALL BE: NON-TRAFFICABLE AREAS: GENERAL 600mm COVER TRAFFICABLE AREAS: SEALED -
- UNLESS STATED OTHERWISE, DECOMMISSIONED PIPES THAT ARE NOT REMOVED AS PART OF THE WORKS SHALL BE CAPPED AND GROUTED IN ACCORDANCE WITH THE WSAA SUPPLY CODE. WHERE THE DISTURBANCE REMOVAL AND/OR CUTTING OF AC PIPE IS REQUIRED. THE CONTRACTOR SHALL REFER TO SWMS FOR THE SAFE MANAGEMENT AND DISPOSAL OF AC PIPE.
- DECOMMISSIONING OF ALL SURFACE FITTINGS/ FIXTURES SHALL INCLUDE THE REMOVAL AND DISPOSAL OF HYDRANTS, VALVE STEMS, BOXES AND MARKERS, AND REINSTATEMENT TO MATCH SURROUNDING AREA.
  ALL EXISTING WATER SERVICES SHALL BE REPLACED UNLESS SHOWN OTHERWISE. SHORT

- TOPSOIL AND SUBSOIL SHALL BE STOCKPILED SEPARATELY.
- CARE SHALL BE TAKEN TO PREVENT SEDIMENT FROM ENTERING THE STORMWATER SYSTEM
- THIS MAY INVOLVE PLACING APPROPRIATE SEDIMENT CONTROLS AROUND STOCKPILES. WHERE ACID SULFATE SOILS HAVE BEEN IDENTIFIED WITHIN THE PROJECTED AREA THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLIANCE WITH ALL RELATED STATUTORY REQUIREMENTS AND SHALL REFER TO SWMS FOR MANAGEMENT AND TREATMENT OF ACID

#### **ENVIRONMENT**

- THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT CONSTRUCTION WORKS DO NOT RESULT IN THE CONTAMINATION OF ADJACENT WATERWAYS, WETLANDS AND/OR ECOSYSTEMS
- THE CONSTRUCTION WORKS WILL BE IMPLEMENTED IN ACCORDANCE WITH EMP AND CONSTRUCTION EMP (C). THIS EMP(C) SHOULD ADDRESS ISSUES AS, BUT NOT LIMITED TO; WATER QUALITY, EROSION AND SEDIMENTATION, CULTURAL HERITAGE, NOISE, VIBRATION AIR QUALITY, ACID SULFATE SOILS. CONTAMINATED SITES, FLORA AND FAUNA, VEGETATION, PEST MANAGEMENT, WASTE, AND CHEMICALS AND FUELS.

#### CONSTRUCTION SPECIFICATIONS

RMS CONSTRUCTION SPECIFICATIONS:

G001 JOB SPECIFIC REQUIREMENTS G002 GENERAL REQUIREMENTS

G010 TRAFFIC MANAGEMENT

G022 WORK HEALTH AND SAFETY G036 ENVIRONMENTAL PROTECTION

G038 SOIL AND WATER MANAGEMENT G040 CLEARING AND GRUBBING

G071 CONSTRUCTION SURVEY
Q03 QUALITY MANAGEMENT SYSTEM

R053 CONCRETE (FOR GENERAL USE)

R044 EARTHWORKS STORMWATER DRAINAGE

KERB AND GUTTERS

UNBOUND & MODIFIED PAVEMENT COURSE INSITU PAVEMENT SATABILISATION

R106 SPRAYED BITUMINOUS SURFACING (CUTBACK)

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R151 STREET LIGHTNING R173 GENERAL CONCRETE PAVING

3051 GRANULAR BASE & SUB-BASE MATERIALS FOR SURFACED ROAD PAVEMENT

SELECTED MATERIAL FOR FORMATION LAYERS 3151 COVER AGGREGATE FOR SPRAYED BITHMINOUS SURFACING

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3268 AGGREGATE PRE-COATING AGENT

3269 BITUMEN ADHESION AGENT

TS106 ITS VIDEO SURVEILLANCE CAMERA SITE

#### WATER SERVICES ASSOCIATION OF AUSTRALIA:

SEWERAGE CODE OF AUSTRALIA

SEWERAGE PUMPING STATION CODE OF AUSTRALIA

NAT-SPEC CONSTRUCTION SPECIFICATIONS:

0292 MASONRY WALLS

0344 STEEL - HDG 0702 MECHANICAL - DESIGN & INSTALL

0802 HYDRAULIC - DESIGN & INSTALL 0902 ELECTRICAL - DESIGN & INSTALL

SERVICE CONDUITS 1392 TRENCHLESS CONDUIT INSTALLATION

#### STANDARD DRAWINGS

BEDDING AND BACKFILL FOR WATER MAIN CONSTRUCTION W-0040

SEWER CONSTRUCTION - PIPELINE CONSTRUCTION TYPES LIFT STATION - SUBMERSIBLE 1800mm DIA

S-0057 PUMP STATION OVERFLOW

G-0041

FENCING - CHAIN WIRE SECURITY FENCING DRAINAGE PITS FIELD INLET - TYPE 1 & 2

TEMPORARY EROSION CONTROL SILT FENCE CONCRETE HEADWALLS SINGLE CELL 300mm TO 900mm DIA WITH CONCRETE APRON. (2 to R-0210-01

INSTALLATION OF BURIED CONCRETE PIPES TYPE HS3 SUPPORT R-0240-01

RURAL ROAD BOUNDARY FENCING - GATE PANEL AND GATE FITTING DETAILS FENCING LOG BARRIER AND ALTERNATIVE HARDWOOD TIMBER BOLLARD

SEQ R-051 DRIVEWAYS - HEAVY DUTY VEHICLE CROSSING

ALL UNDERGROUND SERVICES SHOULD BE LOCATED ON SITE



CONCEPT DESIGN ISSUE

O

CE18119.1

ISSUED FOR APPROVAL

COOTAMUNDRA-GUNDAGAI REGIONAL COUNCIL

**GUNDAGAI - FIVE MILE** HUME HIGHWAY NSW PROP. TRUNK WATER MAIN STG 1



**COVER SHEET & LOCALITY PLAN** 

CE18119.1-001-CO

В



# DRAWING SCHEDULE

**Document Title** 

**Document No.** 

COVER SHEET & LOCALITY PLAN	CE18119.1-001-CO
DRG. SCHEDULE	CE18119.1-002-CO
LEGEND & GENERAL NOTES	CE18119.1-101-GE
GENERAL ARRANGEMENT SHEET 1 OF 9	CE18119.1-201-GA
GENERAL ARRANGEMENT SHEET 2 OF 9	CE18119.1-202-GA
GENERAL ARRANGEMENT SHEET 3 OF 9	CE18119.1-203-GA
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GENERAL ARRANGEMENT SHEET 7 OF 9	CE18119.1-207-GA
GENERAL ARRANGEMENT SHEET 8 OF 9	CE18119.1-208-GA
GENERAL ARRANGEMENT SHEET 9 OF 9	CE18119.1-209-GA

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GUNDAGAI - FIVE MILE HUME HIGHWAY NSW PROP. TRUNK WATER MAIN STG 1



DRG. SCHEDULE

CE18119.1-002-CO

В

# LEGEND **PROPOSED** Proposed Water Service Line

Proposed Trunk Water Main

ŒĐ Proposed Fire Hydrant

P Proposed Flushing Point Proposed Valve

#### **EXISTING SURVEY**

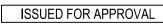
EXISTING SURVET				
	Existing Lot Boundary			
	Permanent Survey Mark			
<u> </u>	Existing Edge of Bitumen			
	Existing Fence Line			
TO/H TO/H	Existing Overhead Communications			
— E—— <sup>O/H</sup> —— E—— <sup>O/H</sup>	Existing Overhead Electrical			
D D	Existing Stormwater Line			
—— w ——— w ———	Existing Water Main (DBYD Location Only)			
s s	Existing Sewer Main (DBYD Location Only)			
NBN	Existing Underground NBN			
	Existing Telstra Pit			
$\oplus$	Existing Power Pole			
	Existing Bitumen Surface			
	Existing Surface Contours			
12.0				

0	2.5	10.0m	1:500 (A1)
0	5.0	 20.0m	1:1000 (A3)

ST JE	CALCS	DRAWN	DATE 11/19	AMENDMENT DETAILS		COPYRIGHT  MOLONEY SOLUTIONS PTY LTD 2019
Α	LM	JO	29/11/19	CONCEPT DESIGN ISSUE		These designs and drawings are copyright and not to be used or reproduced without the written
В	LM	JO	25/05/20	PRELIMINARY DESIGN ISSUE		permission of the above. The contents of this drawing are electronically generated, are confi
					DRAWN CHECK	and may only be used for the purpose for which were intended. This is an uncontrolled docume
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						dimensions take precedence over scale. Do no
						scale from this drawing. Verify dimensions pric commencing any works.



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COOTAMUNDRA-GUNDAGAI REGIONAL COUNCIL

**GUNDAGAI - FIVE MILE HUME HIGHWAY NSW** PROP. TRUNK WATER MAIN STG



**LEGEND & GENERAL NOTES** 

CE18119.1-101-GE

#### **GENERAL:**

- CONSTRUCTION AND MATERIAL SHALL BE UNDERTAKEN AND PROVIDED IN ACCORDANCE WITH THE SYDNEY WATER/WSAA & RMS GUIDELINES.
- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH SYDNEY WATER/WSAA DESIGN & CONSTRUCTION GUIDELINES AND WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ALL DISCREPANCIES SHALL BE REFEREED TO THE SUPERINTENDENT FOR DIRECTION BEFORE PROCEEDING WITH WORKS.
- DIMENSIONS SHALL NOT BE SCALED FROM DRAWINGS
- SET-OUT DIMENSIONS & LEVELS, INCLUDING ANY SHOWN ON THE DRAWINGS ARE TO BE VERIFIED ON SITE BY THE CONTRACTOR BEFORE FABRICATION & CONSTRUCTION.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF PIPES AND PITS AGAINST FLOATATION DURING CONSTRUCTION.
- NO SUBSTITUTE MATERIALS SHALL BE USED WITHOUT THE APPROVAL OF THE SUPERINTENDENT.
- HEIGHT DATUM IS TO AHD & COORDINATES ARE RELATIVE TO MGA94 (ZONE 55).
- SURVEY EXTENTS ARE GENERALLY LIMITED TO THE PROPOSED WATER MAIN ALIGNMENT & DO NOT NECESSARILY INCLUDE ALL EXISTING FEATURES WITHIN THE PROJECT AREA.
- UNDERGROUND SERVICE UTILITIES HAVE BEEN PLOTTED USING SERVICE AUTHORITY RECORDS & MAY NOT REPRESENT THE ACTUAL SERVICE LOCATION. WHERE REQUIRED, THE PLOTTED LOCATION OF THESE SERVICES MAY HAVE BEEN ALTERED TO FIT ACTUAL SURVEYED
- BEDDING TO CONFORM TO AUS-SPEC, 1341 WATER SUPPLY & 1361 SEWER SYSTEM SPECIFICATIONS;
- SPECIAL BEDDING CONDITIONS TO BE SPECIFIED BY THE PRINCIPALS AUTHORIZED PERSON TO SUIT THE CONDITIONS IF THE TRENCH FLOOR HAS:
- IRREGULAR OUTCROPS OF ROCK.
- AHBP OF <50kPa (SEE SEW-1200), OR
- BEEN DISTURBED BY UNCONTROLLED GROUND WATER.
- COMPACT AND EVENLY GRADE FINISHED TRENCH FLOOR.
- EMBEDMENT, TRENCH FILL AND COMPACTION TO MEET THE REQUIREMENT OF AUS-SPEC 1341, 1362, DESIGN DRAWINGS & WSA 02 PART 3.
- USE GEOTEXTILE FILTER FABRIC WHERE SPECIFIED.
- SIDES OF EXCAVATION TO BE KEPT VERTICAL TO AT LEAST 150 ABOVE THE PIPE

#### PROPOSED WATER WORKS:

- WHERE CONNECTION TO EXISTING PIPEWORK, THE LEVEL & DIAMETER OF THE EXISTING PIPEWORK SHALL BE CONFIRMED BY THE CONTRACTOR PRIOR TO CONNECTION.
- CHANGES IN HORIZONTAL & VERTICAL ALIGNMENT, OTHER THAN THROUGH FABRICATED BENDS, SHALL BE ACHIEVED BY DEFLECTION AT PIPE JOINTS WHERE SHOWN. MAXIMUM DEFLECTION SHALL BE AS PER MANUFACTURERS SPECIFICATIONS.
- PROVIDE SUFFICIENT TRENCH DEPTH AND WIDTH TO ALLOW FOR DEFLECTION OF PIPES AT JOINT'S.
- TEST PRESSURE SHALL BE 1200kpa AT THE LOWEST POINT ON THE PIPELINE.
- ALL PIPE FITTINGS SHALL BE PE100 MIN.
- PRESSURE MAIN CLEARANCE FROM ALL EXISTING SERVICES SHALL B EIN ACCORDANCE WITH SYDNEY WATER/WSA GUIDELINES. IF CLEARANCE BETWEEN SERVICES IS LESS THAN THAT SPECIFIED, REFER TO THE SUPERINTENDENT FOR DIRECTION BEFORE PROCEEDING
- MIN. PRESSURE MAIN COVER FROM FINISHED GROUND LEVEL TO THE OBVERT OF THE MAIN SHALL BE;
- 600mm Cover Non-trafficable areas; &
- 600mm Cover Trafficable areas
- UNLESS STATED OTHERWISE, DECOMMISSIONED PIPES THAT ARE NOT REMOVED AS PART OF THE WORKS SHALL BE CAPPED AND GROUTED IN ACCORDANCE WITH THE SYDNEY WATER/WSAA GUIDELINES. WHERE THE DISTURBANCE, REMOVAL AND/OR CUTTING OF AC PIPE IS REQUIRED. THE CONTRACTOR SHALL REFER TO SWMS FOR THE SAFE MANAGEMENT AND DISPOSAL OF AC PIPE.
- DECOMMISSIONG OF ALL SURFACE FITTING/FIXTURES SHALL INCLUDE THE REMOVAL AND DISPONAL OF HYDRANTS, VALVE STEMS, BOXES AND MARKERS AND REINSTATEMENT TO MATCH SURROUNDING AREA.
- ALL EXISTING WATER SERVICES SHALL BE REPLACED UNLESS SHOWN OTHERWISE. SHORT WATER SERVICE CONNECTION OMITTED FOR

#### **EARTHWORKS:**

- TOPSOIL AND SUBSOIL SHALL BE STOCKPILED SEPARATELY.
- CARE SHALL BE TAKEN TO PREVENT SEDIMENT FROM ENTERING THE SYSTEM. THIS MAY INVOLVE PLACING APPROPRIATE SEDIMENT CONTROLS AROUND STOCKPILES.
- WHERE ACID SULPHATE SOILS HAVE BEEN IDENTIFIED WITHIN THE PROJECT AREA. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLIANCE WITH ALL RELATED STATUTORY REQUIREMENTS AND SHALL REFER TO SWMS FOR MANAGEMENT AND TREATMENT OF ACID SULPHATE SOILS.

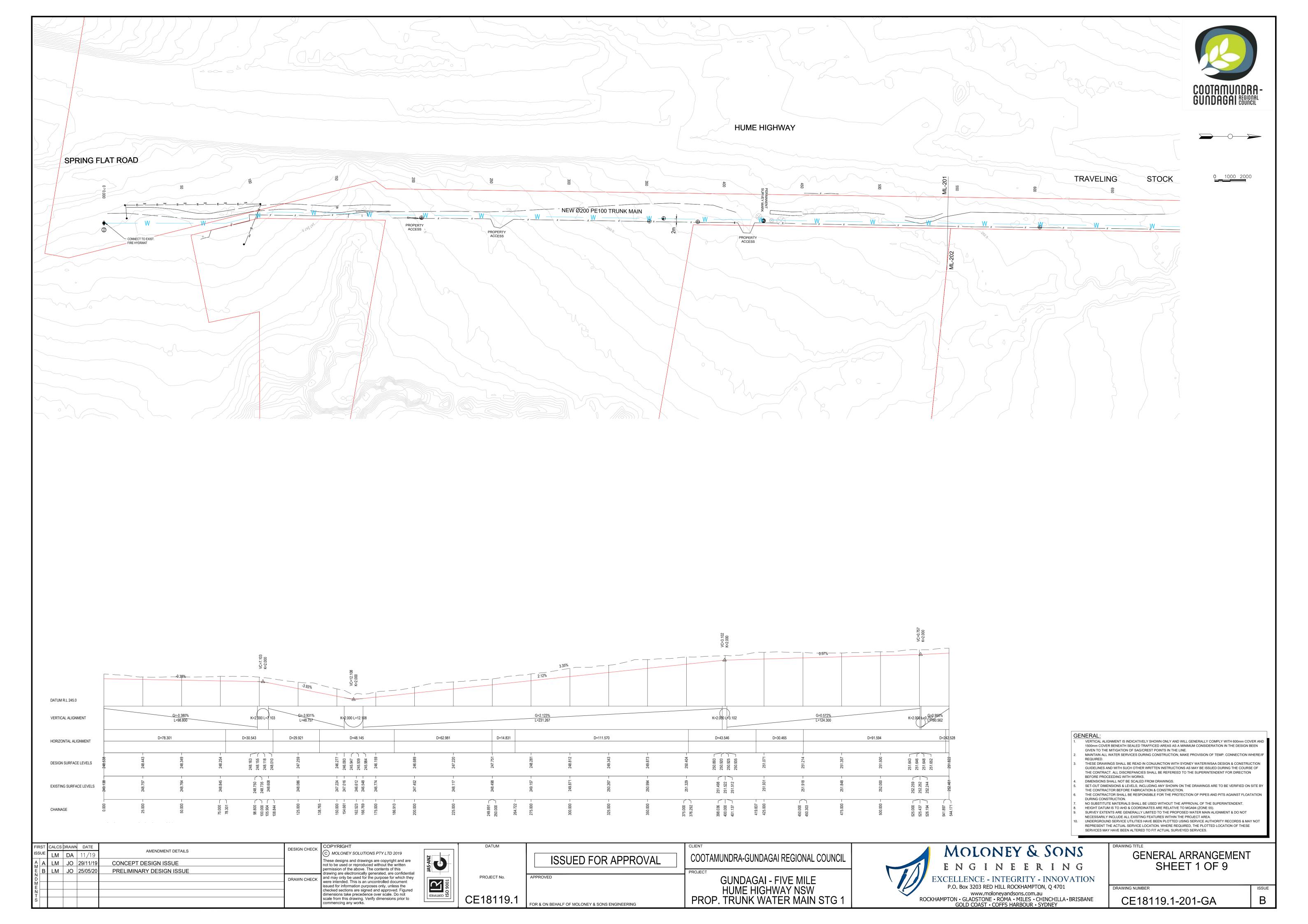
#### **ENVIRONMENT:**

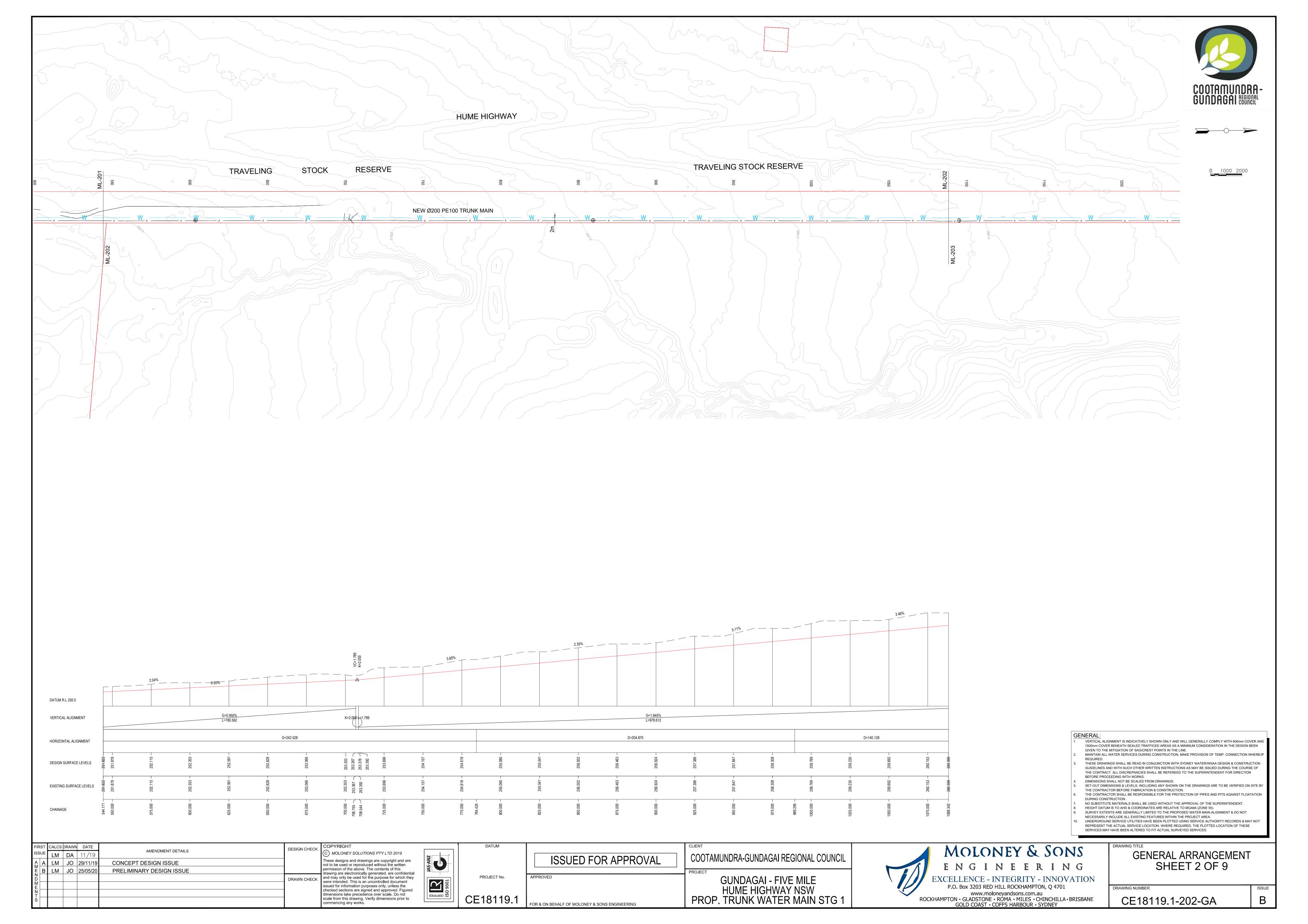
- THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT CONSTRUCTION WORKS DO NOT RESULT IN THE CONTAMINATION OF ADJACENT WATERWAYS, WETLANDS AND/OR ECOSYSTEMS.
- THE CONSTRUCTION WORKS WILL BE IMPLEMENTED IN ACCORDANCE WITH EMP & CONSTRUCTION EMP(C). THIS EMP(C) SHOULD ADDRESS ISSUES SUCH AS, BUT NOT LIMITED TO; WATER QUALITY, EROSION AND SEDIMENTATION, CULTURAL HERITAGE, NOISE, VIBRATION, AIR QUALITY, ACID SULPHATE SOILS, CONTAMINATED SITES, FLORA & FAUNA, VEGETATION, PEST MANAGEMENT, WASTE AND CHEMICAL/FUELS.

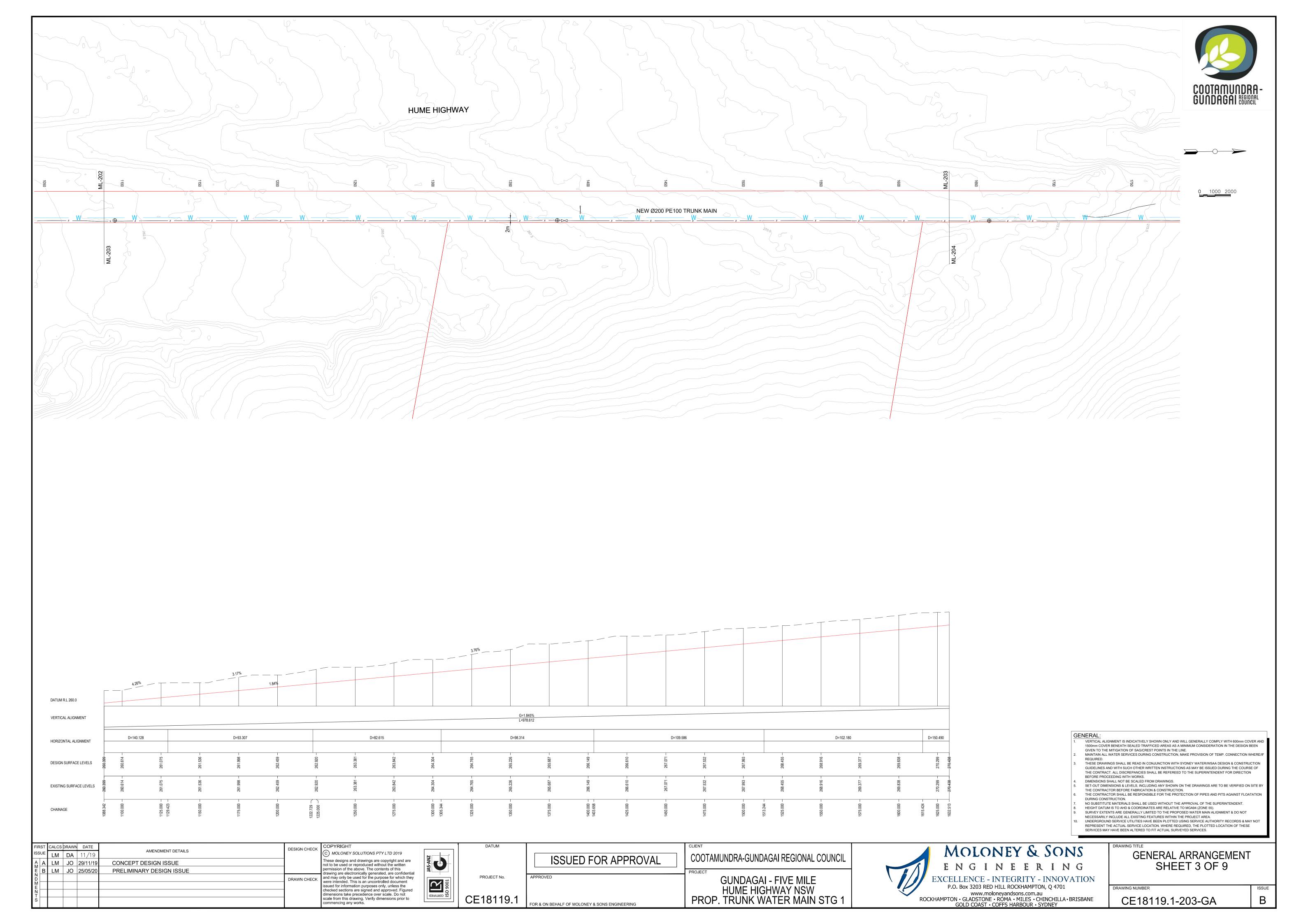
### REINSTATEMENT:

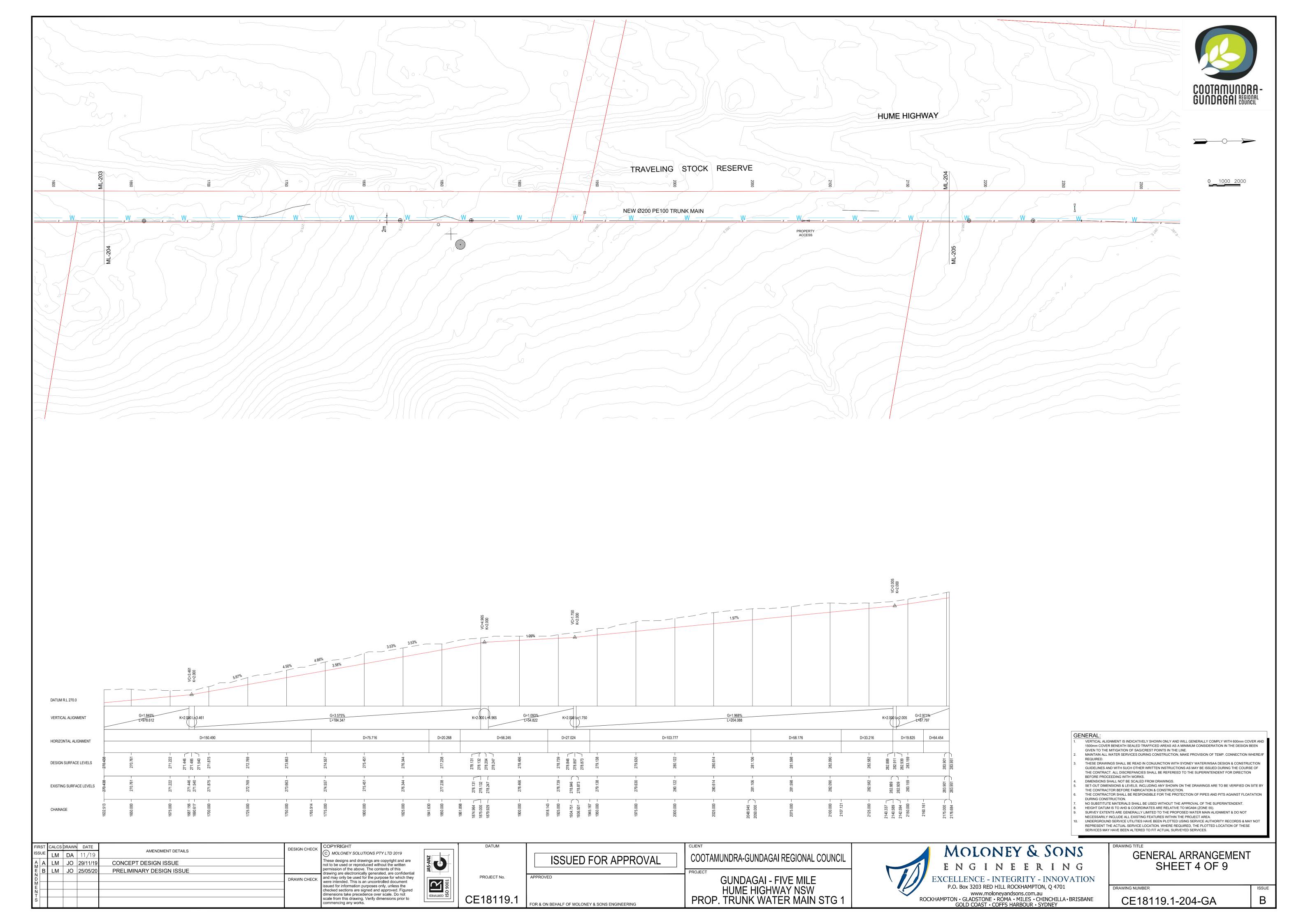
UNLESS STATED OTHERWISE ALL EXISTING INFRASTRUCTURE AFFECTED BY THESE WORKS SHALL BE REINSTATED IN ACCORDANCE WITHE THE RELEVANT STANDARD SPECIFIED IN THE COOTAMUNDRA-GUNDAGAI REGIONAL COUNCIL GUIDELINES. THE FOLLOWING REINSTATEMENT REQUIREMENTS SHALL BE READ IN CONJUNCTION WITH THE PROJECT SPECIFICATIONS.

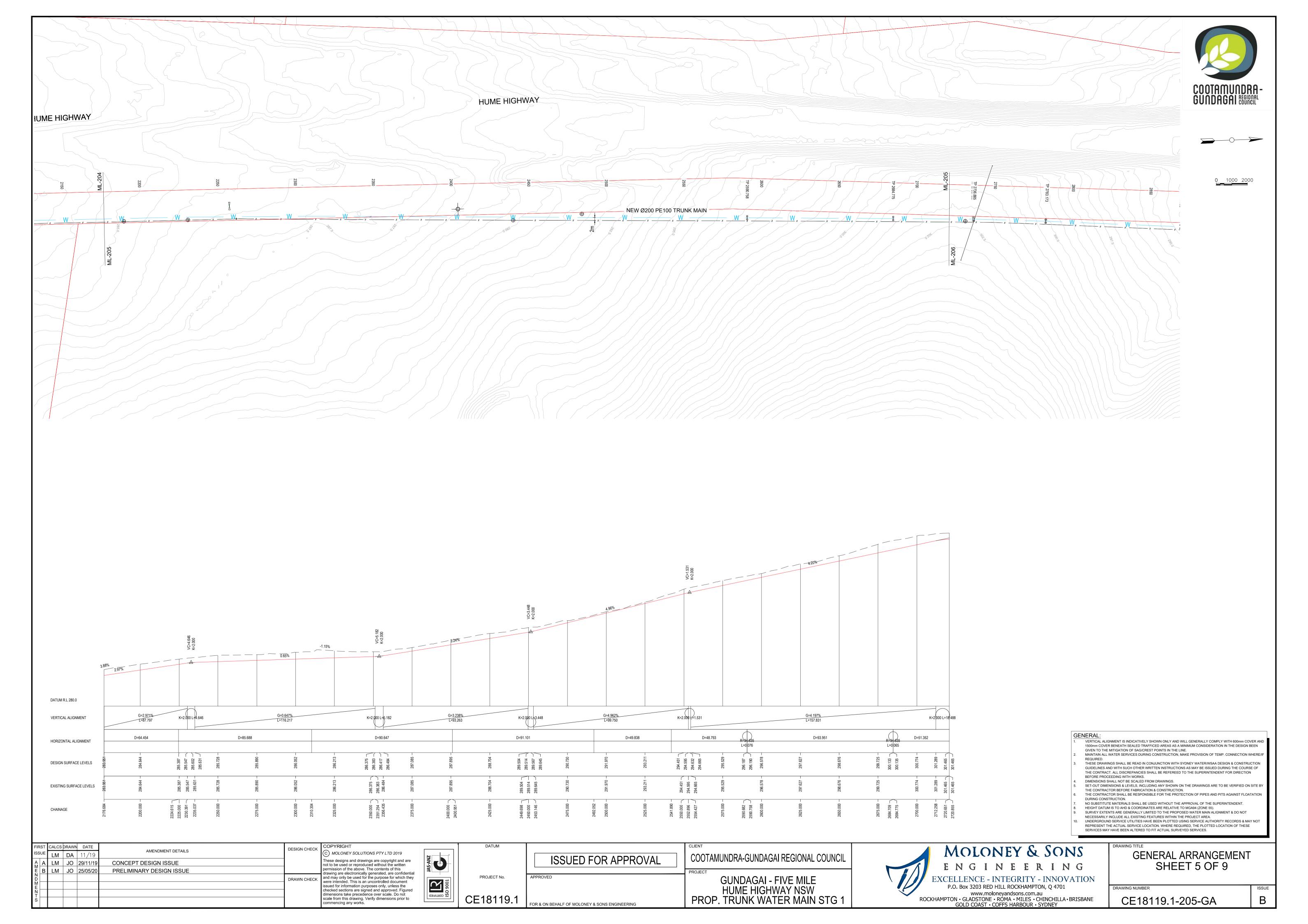
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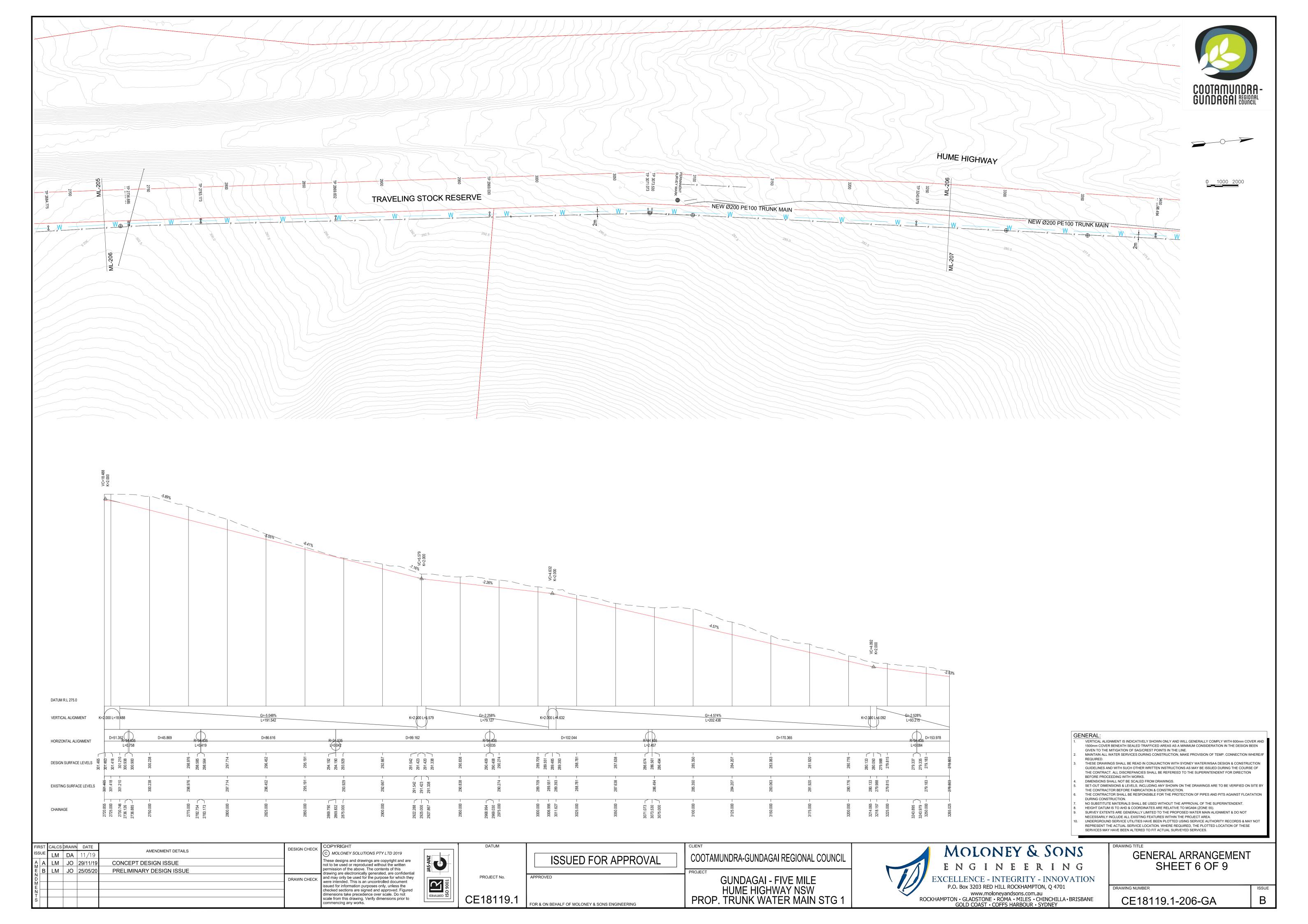


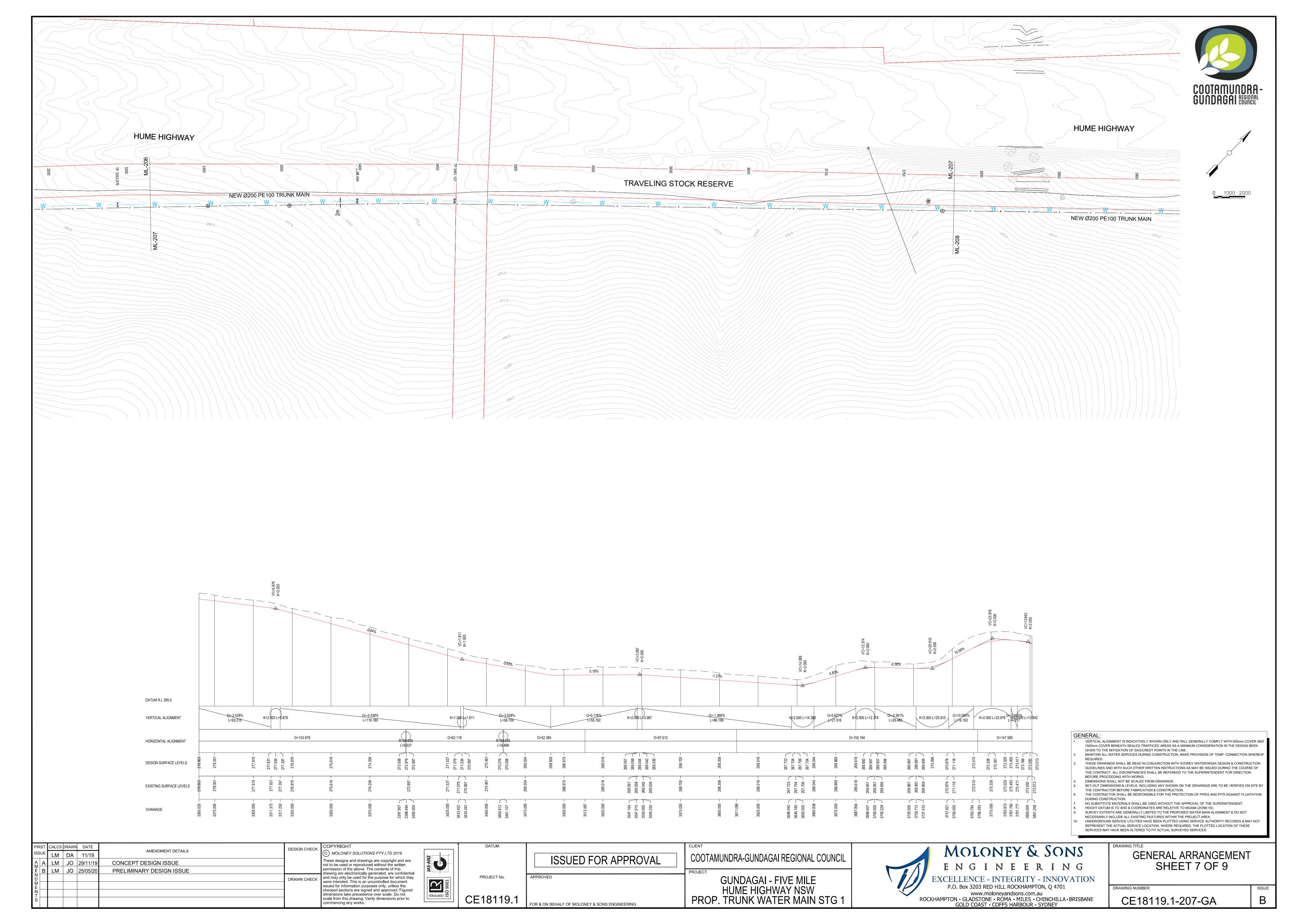


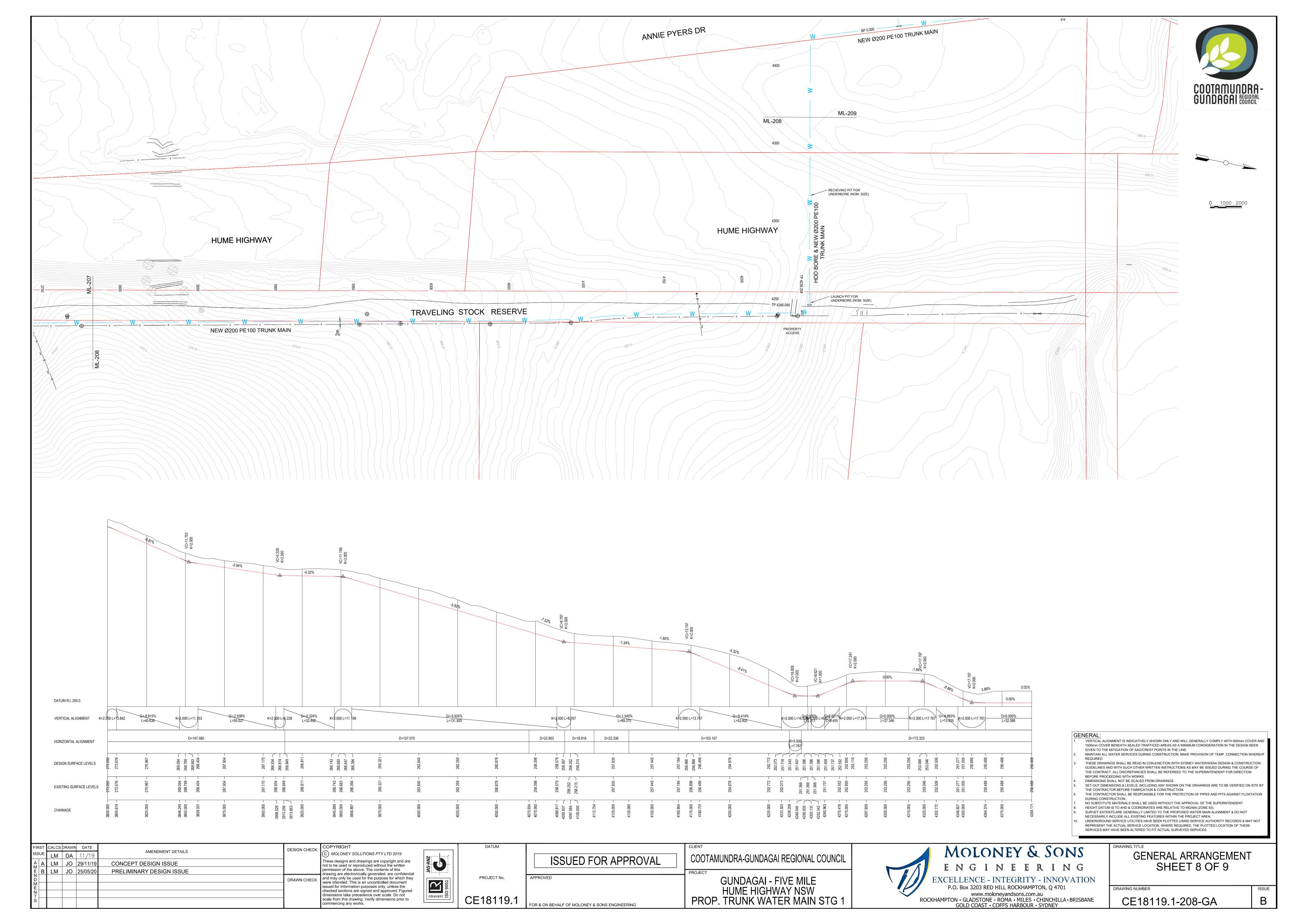


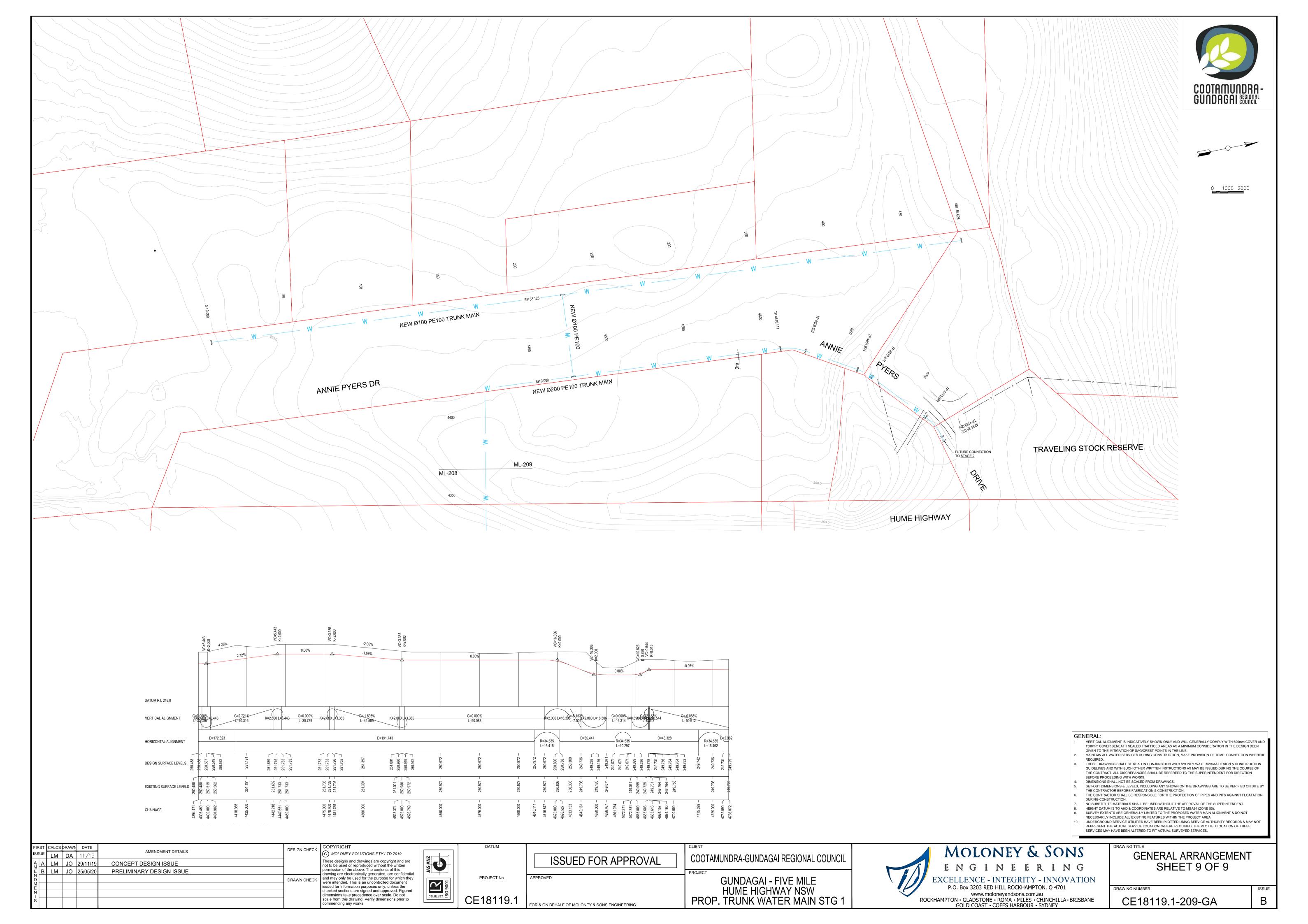










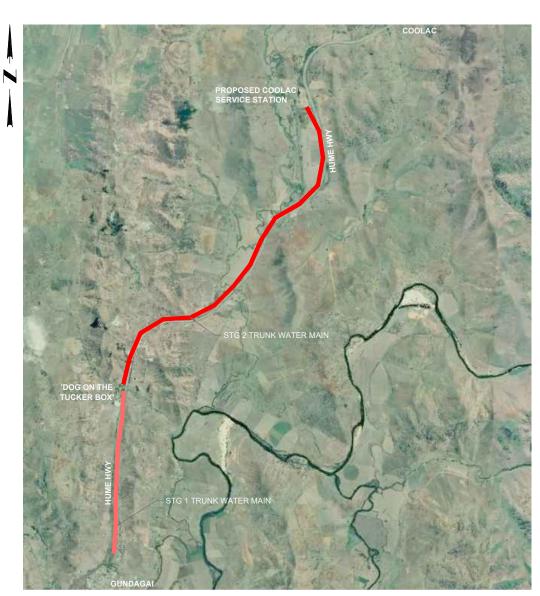


# COOTAMUNDRA-GUNDAGAI REGIONAL COUNCIL

# 'DTB' FIVE MILE TO COOLAC TRUNK WATER MAIN

# STAGE 2





LOCALITY PLAN NOT TO SCALE

DETAIL SURVEY BY: ALLSPEC & PARTNERS - DATE SURVEYED 11/07/18 SURVEY: MGA94 ZONE 55 STANDARD DRAWINGS:

CGRC STANDARD DESIGN DRAWINGS, SYDNEY WATER STD DRAWINGS & SPECIFICATIONS, RMS GUIDELINES & INSTITUTE OF PUBLIC WORKS ENGINEERING AUSTRALIA (IPWEA)

#### PROPOSED TRUNK WATER MAIN - 2019

DESIGN FILE No: CE18119

DESIGN STANDARD: CGRC STD DRGS, AUSTROADS & RMS DESIGN GUIDELINES

#### **GENERAL**

- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONSULTANTS DRAWINGS AND SPECIFICATIONS
- BEFORE PROCEEDING WITH THE WORK ANY DISCREPANCIES IN THE CONTRACT DOCUMENTS SHALL BE REFERRED FOR DECISION TO THE SUPERINTENDENT
- DO NOT SCALE FROM DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL LOCATIONS OF SERVICES, ALL DIMENSIONS AND LEVELS PRIOR TO CONSTRUCTION.
- ALL MATERIALS/CONSTRUCTION & WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE CONTRACT SPECIFICATIONS AND THE LOCAL AUTHORITY'S STANDARD DRAWINGS AND BY-LAWS.
- THE CONTRACTOR IS RESPONSIBLE TO OBTAIN ALL RELEVANT APPROVALS PRIOR TO COMMENCEMENT OF WORKS
- UNDERGROUND SERVICE LOCATIONS SHOWN ON THIS PLAN HAVE BEEN DETERMINED BY FIELD SURVEY AND/OR OFFICE RECORDS, AND MAY NOT REPRESENT ALL SERVICES OR EXACT LOCATIONS. THE CONTRACTOR MUST ACCURATELY LOCATE AND DEPTH ALL SERVICES LIKELY TO BE ENCOUNTERED DURING CONSTRUCTION. PRIOR TO COMMENCING ANY EXCAVATION WORKS
- DISPERSIVE SOILS ARE NOT TO BE USED AS FILL/EMBANKMENT MATERIAL. ALL CONSTRUCTION TO BE DONE IN ACCORDANCE WITH CURRENT RMS SPECIFICATIONS AND TECHNICAL STANDARDS

#### WATER & WASTE WATER

- CONSTRUCTION AND MATERIALS SHALL BE UNDERTAKEN AND PROVIDED IN ACCORDANCE WITH THE WSAA SUPPLY CODE.
- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE WSAA SUPPLY CODE & DRAWINGS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ALL DISCREPANCIES SHALL BE REFERRED TO THE SUPERINTENDENT FOR DIRECTION BEFORE PROCEEDING WITH WORK.
- DIMENSIONS SHALL NOT BE SCALED FROM DRAWINGS.
- SET-OUT DIMENSIONS AND LEVELS, INCLUDING ANY SHOWN ON THE DRAWING ARE TO BE
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  THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF PIPES AND PITS AGAINST FLOATATION DURING CONSTRUCTION.
  NO SUBSTITUTE MATERIAL SHALL BE USED WITHOUT APPROVAL OF THE SUPERINTENDENT.
- HEIGHT DATUM IS TO AHD AND COORDINATES ARE RELATIVE TO MGA94 (ZONE56).
- SURVEY EXTENTS ARE GENERALLY LIMITED TO THE PROPOSED WATER MAIN ALIGNMENT AND DO NOT NECESSARILY INCLUDE ALL EXISTING FEATURES WITHIN THE PROJECT AREA.
- UNDERGROUND SERVICE UTILITIES HAVE BEEN PLOTTED USING SERVICE AUTHORITY RECORDS AND MAY NOT REPRESENT THE ACTUAL SERVICE LOCATION. WHERE REQUIRED, THE PLOTTED LOCATION OF THESE SERVICES MAY HAVE BEEN ALTERED TO FIT ACTUAL SURVEYED SERVICES

- WHERE CONNECTING TO EXISTING PIPEWORK, THE LEVEL AND DIAMETER OF THE EXISTING PIPEWORK SHALL BE CONFIRMED BY THE CONTRACTOR PRIOR TO CONNECTION.
- CHANGES IN HORIZONTAL AND VERTICAL ALIGNMENT, OTHER THAN THROUGH FABRICATED BENDS, SHALL BE ACHIEVED BY DEFLECTION AT PIPE JOINTS WHERE SHOWN. MAXIMUM DEFLECTION SHALL BE AS PER MANUFACTURER'S SPECIFICATIONS.
- PROVIDE SUFFICIENT TRENCH DEPTH AND WIDTH TO ALLOW FOR DEFLECTION OF PIPES AT
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- MINIMUM PRESSURE MAIN COVER FROM FINISHED GROUND LEVEL TO THE OBVERT OF THE MAIN SHALL BE: NON-TRAFFICABLE AREAS: GENERAL 600mm COVER TRAFFICABLE AREAS: SEALED -
- UNLESS STATED OTHERWISE, DECOMMISSIONED PIPES THAT ARE NOT REMOVED AS PART OF THE WORKS SHALL BE CAPPED AND GROUTED IN ACCORDANCE WITH THE WSAA SUPPLY CODE. WHERE THE DISTURBANCE REMOVAL AND/OR CUTTING OF AC PIPE IS REQUIRED. THE CONTRACTOR SHALL REFER TO SWMS FOR THE SAFE MANAGEMENT AND DISPOSAL OF AC PIPE.
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#### **ENVIRONMENT**

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RMS CONSTRUCTION SPECIFICATIONS:

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G010 TRAFFIC MANAGEMENT

G022 WORK HEALTH AND SAFETY G036 ENVIRONMENTAL PROTECTION

G038 SOIL AND WATER MANAGEMENT G040 CLEARING AND GRUBBING

G071 CONSTRUCTION SURVEY
Q03 QUALITY MANAGEMENT SYSTEM

R053 CONCRETE (FOR GENERAL USE)

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1392 TRENCHLESS CONDUIT INSTALLATION

#### STANDARD DRAWINGS

BEDDING AND BACKFILL FOR WATER MAIN CONSTRUCTION W-0040 SEWER CONSTRUCTION - PIPELINE CONSTRUCTION TYPES

S-0057 LIFT STATION - SUBMERSIBLE 1800mm DIA

PUMP STATION OVERFLOW

G-0041 FENCING - CHAIN WIRE SECURITY FENCING DRAINAGE PITS FIELD INLET - TYPE 1 & 2

TEMPORARY EROSION CONTROL SILT FENCE CONCRETE HEADWALLS SINGLE CELL 300mm TO 900mm DIA WITH CONCRETE APRON. (2 to R-0210-01

INSTALLATION OF BURIED CONCRETE PIPES TYPE HS3 SUPPORT R-0240-01

RURAL ROAD BOUNDARY FENCING - GATE PANEL AND GATE FITTING DETAILS

FENCING LOG BARRIER AND ALTERNATIVE HARDWOOD TIMBER BOLLARD

SEQ R-051 DRIVEWAYS - HEAVY DUTY VEHICLE CROSSING

ALL UNDERGROUND SERVICES SHOULD BE LOCATED ON SITE

BEFORE YOU DIG

				■ AMENDMENT DETAILS					
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ISSUED FOR APPROVAL

COOTAMUNDRA-GUNDAGAI REGIONAL COUNCIL

FIVE MILE - COOLAC SERVICE STN **HUME HIGHWAY NSW** PROP. TRUNK WATER MAIN STG 2



**COVER SHEET & LOCALITY PLAN** 

ROAD PAVEMENT

CE18119.2-001-CO





# DRAWING SCHEDULE

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GENERAL ARRANGEMENT SHEET 1 OF 20	CE18119.2-201-GA
GENERAL ARRANGEMENT SHEET 2 OF 20	CE18119.2-202-GA
GENERAL ARRANGEMENT SHEET 3 OF 20	CE18119.2-203-GA
GENERAL ARRANGEMENT SHEET 4 OF 20	CE18119.2-204-GA
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GENERAL ARRANGEMENT SHEET 6 OF 20	CE18119.2-206-GA
GENERAL ARRANGEMENT SHEET 7 OF 20	CE18119.2-207-GA
GENERAL ARRANGEMENT SHEET 8 OF 20	CE18119.2-208-GA
GENERAL ARRANGEMENT SHEET 9 OF 20	CE18119.2-209-GA
GENERAL ARRANGEMENT SHEET 10 OF 20	CE18119.2-210-GA
GENERAL ARRANGEMENT SHEET 11 OF 20	CE18119.2-211-GA
GENERAL ARRANGEMENT SHEET 12 OF 20	CE18119.2-212-GA
GENERAL ARRANGEMENT SHEET 13 OF 20	CE18119.2-213-GA
GENERAL ARRANGEMENT SHEET 14 OF 20	CE18119.2-214-GA
GENERAL ARRANGEMENT SHEET 15 OF 20	CE18119.2-215-GA
GENERAL ARRANGEMENT SHEET 16 OF 20	CE18119.2-216-GA
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CE18119.2

ISSUED FOR APPROVAL

COOTAMUNDRA-GUNDAGAI REGIONAL COUNCIL

FIVE MILE - COOLAC SERVICE STN HUME HIGHWAY NSW PROP. TRUNK WATER MAIN STG 2



DRG. SCHEDULE

CE18119.2-002-CO

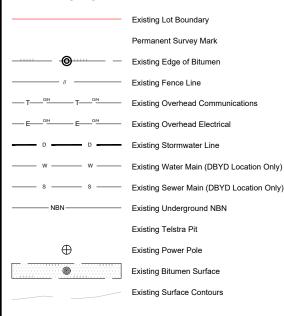
## **LEGEND PROPOSED** Proposed Trunk Water Main Proposed Water Service Line $\oplus$

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Proposed Flushing Point Proposed Valve

Proposed Fire Hydrant

#### **EXISTING SURVEY**



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COOTAMUNDRA-GUNDAGAI REGIONAL COUNCIL



**LEGEND & GENERAL NOTES** 

CE18119.2-101-GE



#### **GENERAL:**

- CONSTRUCTION AND MATERIAL SHALL BE UNDERTAKEN AND PROVIDED IN ACCORDANCE WITH THE SYDNEY WATER/WSAA & RMS GUIDELINES.
- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH SYDNEY WATER/WSAA DESIGN & CONSTRUCTION GUIDELINES AND WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ALL DISCREPANCIES SHALL BE REFEREED TO THE SUPERINTENDENT FOR DIRECTION BEFORE PROCEEDING WITH WORKS.
- DIMENSIONS SHALL NOT BE SCALED FROM DRAWINGS.
- SET-OUT DIMENSIONS & LEVELS, INCLUDING ANY SHOWN ON THE DRAWINGS ARE TO BE VERIFIED ON SITE BY THE CONTRACTOR BEFORE FABRICATION & CONSTRUCTION.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF PIPES AND PITS AGAINST FLOATATION DURING CONSTRUCTION.
- NO SUBSTITUTE MATERIALS SHALL BE USED WITHOUT THE APPROVAL OF THE SUPERINTENDENT.
- HEIGHT DATUM IS TO AHD & COORDINATES ARE RELATIVE TO MGA94 (ZONE 55).
- SURVEY EXTENTS ARE GENERALLY LIMITED TO THE PROPOSED WATER MAIN ALIGNMENT & DO NOT NECESSARILY INCLUDE ALL EXISTING FEATURES WITHIN THE PROJECT AREA.
- UNDERGROUND SERVICE UTILITIES HAVE BEEN PLOTTED USING SERVICE AUTHORITY RECORDS & MAY NOT REPRESENT THE ACTUAL SERVICE LOCATION. WHERE REQUIRED, THE PLOTTED LOCATION OF THESE SERVICES MAY HAVE BEEN ALTERED TO FIT ACTUAL SURVEYED SERVICES.
- BEDDING TO CONFORM TO AUS-SPEC, 1341 WATER SUPPLY & 1361 SEWER SYSTEM SPECIFICATIONS
- SPECIAL BEDDING CONDITIONS TO BE SPECIFIED BY THE PRINCIPALS AUTHORIZED PERSON TO SUIT THE CONDITIONS IF THE TRENCH FLOOR HAS:
- IRREGULAR OUTCROPS OF ROCK.
- AHBP OF <50kPa (SEE SEW-1200), OR
- BEEN DISTURBED BY UNCONTROLLED GROUND WATER.
- COMPACT AND EVENLY GRADE FINISHED TRENCH FLOOR.
- EMBEDMENT, TRENCH FILL AND COMPACTION TO MEET THE REQUIREMENT OF AUS-SPEC 1341, 1362, DESIGN DRAWINGS & WSA 02 PART 3.
- USE GEOTEXTILE FILTER FABRIC WHERE SPECIFIED.
- SIDES OF EXCAVATION TO BE KEPT VERTICAL TO AT LEAST 150 ABOVE THE PIPE

#### PROPOSED WATER WORKS:

- WHERE CONNECTION TO EXISTING PIPEWORK, THE LEVEL & DIAMETER OF THE EXISTING PIPEWORK SHALL BE CONFIRMED BY THE CONTRACTOR PRIOR TO CONNECTION.
- CHANGES IN HORIZONTAL & VERTICAL ALIGNMENT, OTHER THAN THROUGH FABRICATED BENDS, SHALL BE ACHIEVED BY DEFLECTION AT PIPE JOINTS WHERE SHOWN. MAXIMUM DEFLECTION SHALL BE AS PER MANUFACTURERS SPECIFICATIONS.
- PROVIDE SUFFICIENT TRENCH DEPTH AND WIDTH TO ALLOW FOR DEFLECTION OF PIPES AT JOINT'S.
- TEST PRESSURE SHALL BE 1200kpa AT THE LOWEST POINT ON THE PIPELINE.
- ALL PIPE FITTINGS SHALL BE PE100 MIN.
- PRESSURE MAIN CLEARANCE FROM ALL EXISTING SERVICES SHALL B EIN ACCORDANCE WITH SYDNEY WATER/WSA GUIDELINES. IF CLEARANCE BETWEEN SERVICES IS LESS THAN THAT SPECIFIED, REFER TO THE SUPERINTENDENT FOR DIRECTION BEFORE PROCEEDING
- MIN. PRESSURE MAIN COVER FROM FINISHED GROUND LEVEL TO THE OBVERT OF THE MAIN SHALL BE;
- 600mm Cover Non-trafficable areas; &
- 600mm Cover Trafficable areas
- UNLESS STATED OTHERWISE, DECOMMISSIONED PIPES THAT ARE NOT REMOVED AS PART OF THE WORKS SHALL BE CAPPED AND GROUTED IN ACCORDANCE WITH THE SYDNEY WATER/WSAA GUIDELINES. WHERE THE DISTURBANCE, REMOVAL AND/OR CUTTING OF AC PIPE IS REQUIRED. THE CONTRACTOR SHALL REFER TO SWMS FOR THE SAFE MANAGEMENT AND DISPOSAL OF AC PIPE.
- DECOMMISSIONG OF ALL SURFACE FITTING/FIXTURES SHALL INCLUDE THE REMOVAL AND DISPONAL OF HYDRANTS, VALVE STEMS, BOXES AND MARKERS AND REINSTATEMENT TO MATCH SURROUNDING AREA.
- ALL EXISTING WATER SERVICES SHALL BE REPLACED UNLESS SHOWN OTHERWISE. SHORT WATER SERVICE CONNECTION OMITTED FOR

#### **EARTHWORKS:**

- TOPSOIL AND SUBSOIL SHALL BE STOCKPILED SEPARATELY.
- CARE SHALL BE TAKEN TO PREVENT SEDIMENT FROM ENTERING THE SYSTEM. THIS MAY INVOLVE PLACING APPROPRIATE SEDIMENT CONTROLS AROUND STOCKPILES.
- WHERE ACID SULPHATE SOILS HAVE BEEN IDENTIFIED WITHIN THE PROJECT AREA. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLIANCE WITH ALL RELATED STATUTORY REQUIREMENTS AND SHALL REFER TO SWMS FOR MANAGEMENT AND TREATMENT OF ACID SULPHATE SOILS.

#### **ENVIRONMENT:**

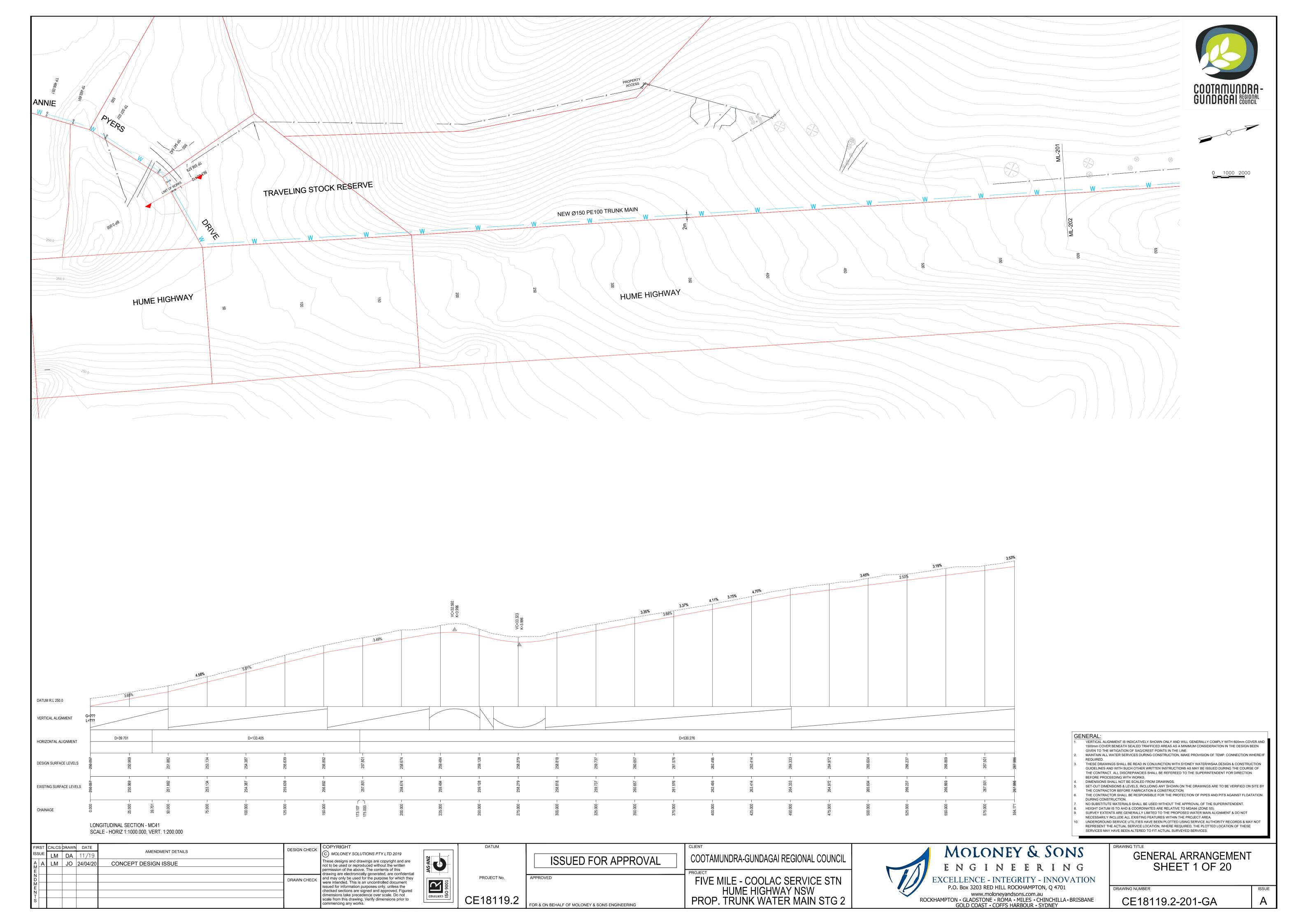
- THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT CONSTRUCTION WORKS DO NOT RESULT IN THE CONTAMINATION OF ADJACENT WATERWAYS, WETLANDS AND/OR ECOSYSTEMS.
- THE CONSTRUCTION WORKS WILL BE IMPLEMENTED IN ACCORDANCE WITH EMP & CONSTRUCTION EMP(C). THIS EMP(C) SHOULD ADDRESS ISSUES SUCH AS, BUT NOT LIMITED TO; WATER QUALITY, EROSION AND SEDIMENTATION, CULTURAL HERITAGE, NOISE, VIBRATION, AIR QUALITY, ACID SULPHATE SOILS, CONTAMINATED SITES, FLORA & FAUNA, VEGETATION, PEST MANAGEMENT, WASTE AND CHEMICAL/FUELS.

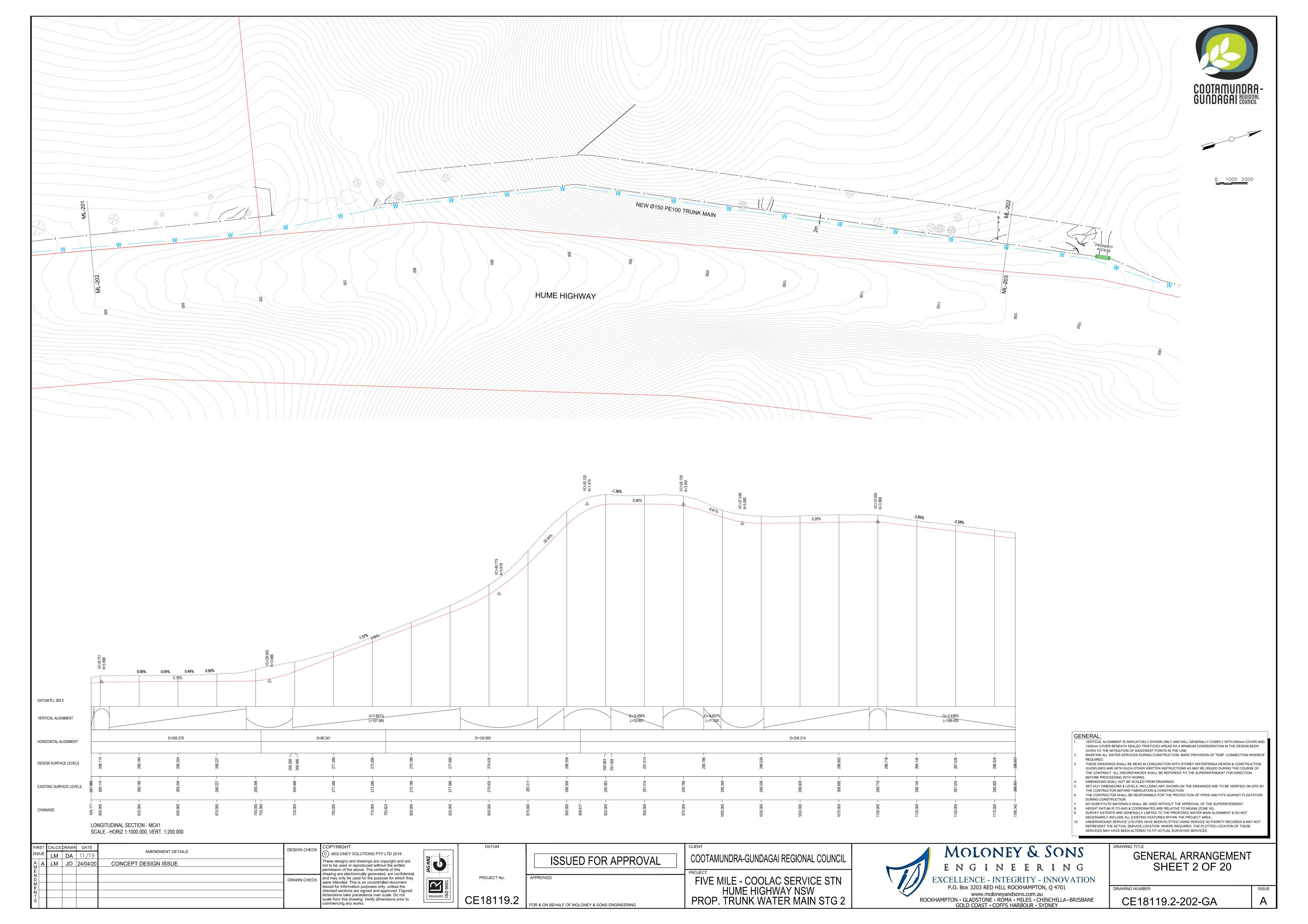
## REINSTATEMENT:

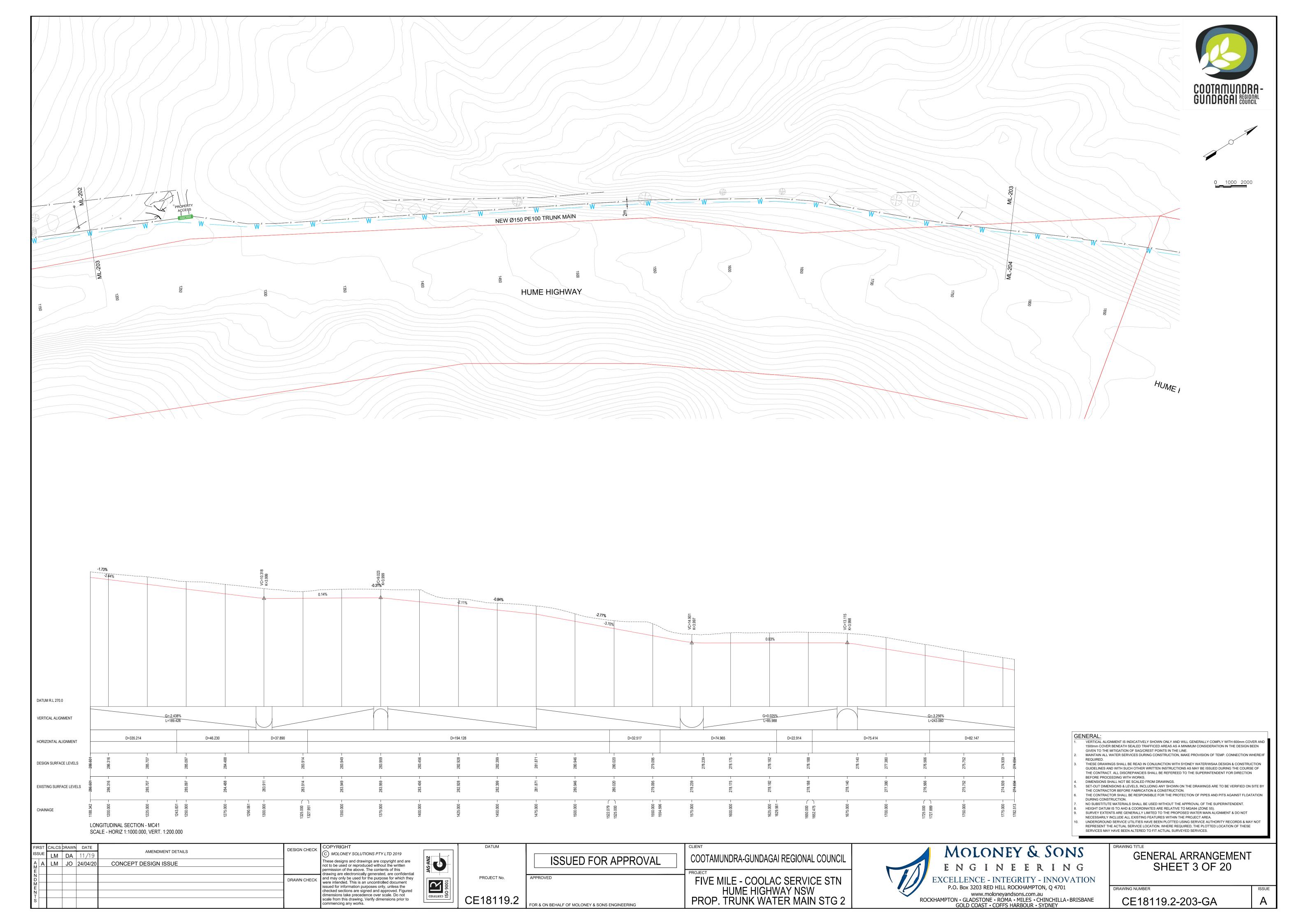
UNLESS STATED OTHERWISE ALL EXISTING INFRASTRUCTURE AFFECTED BY THESE WORKS SHALL BE REINSTATED IN ACCORDANCE WITHE THE RELEVANT STANDARD SPECIFIED IN THE COOTAMUNDRA-GUNDAGAI REGIONAL COUNCIL GUIDELINES. THE FOLLOWING REINSTATEMENT REQUIREMENTS SHALL BE READ IN CONJUNCTION WITH THE PROJECT SPECIFICATIONS.

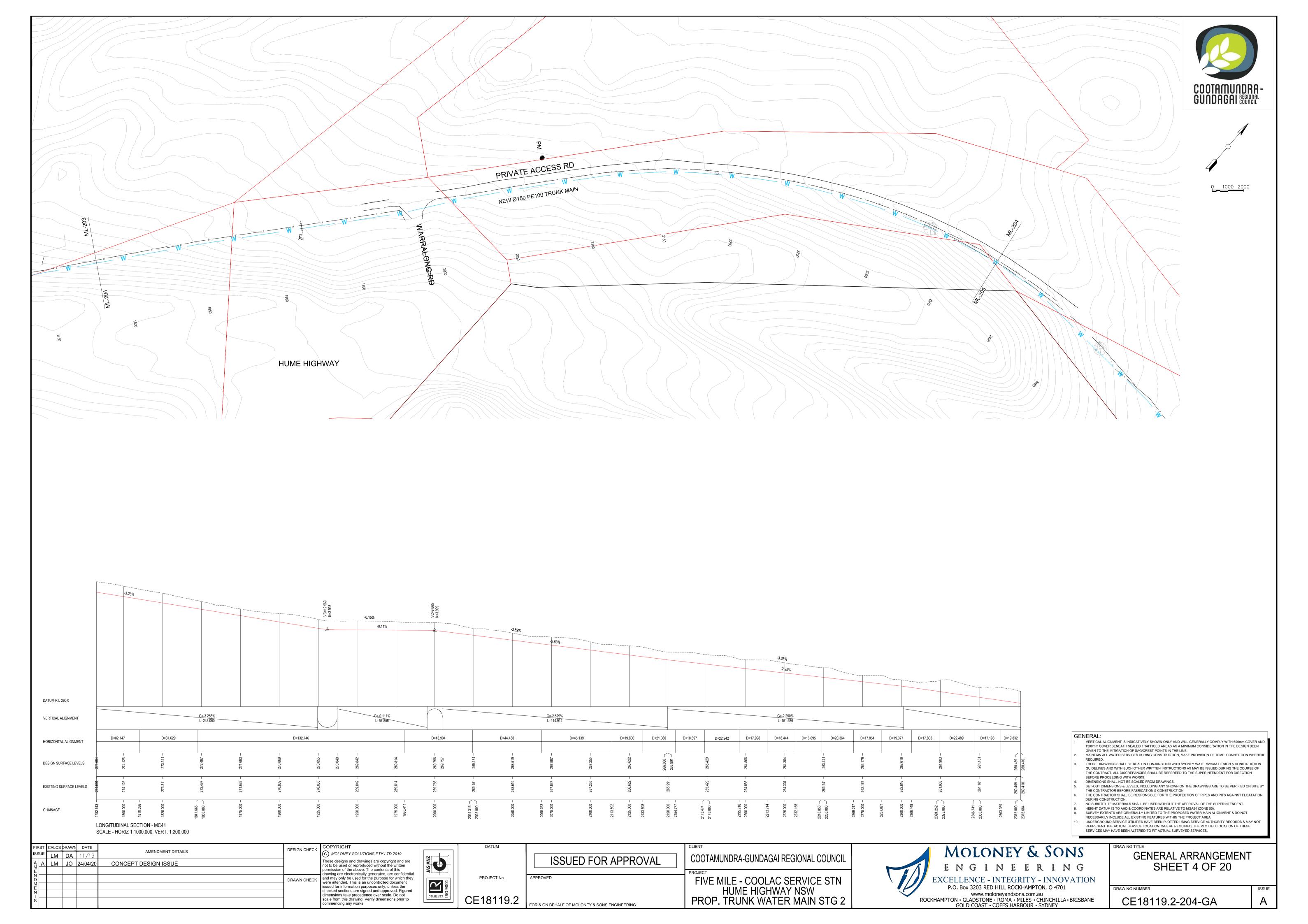
OR & ON BEHALF OF MOLONEY & SONS ENGINEER

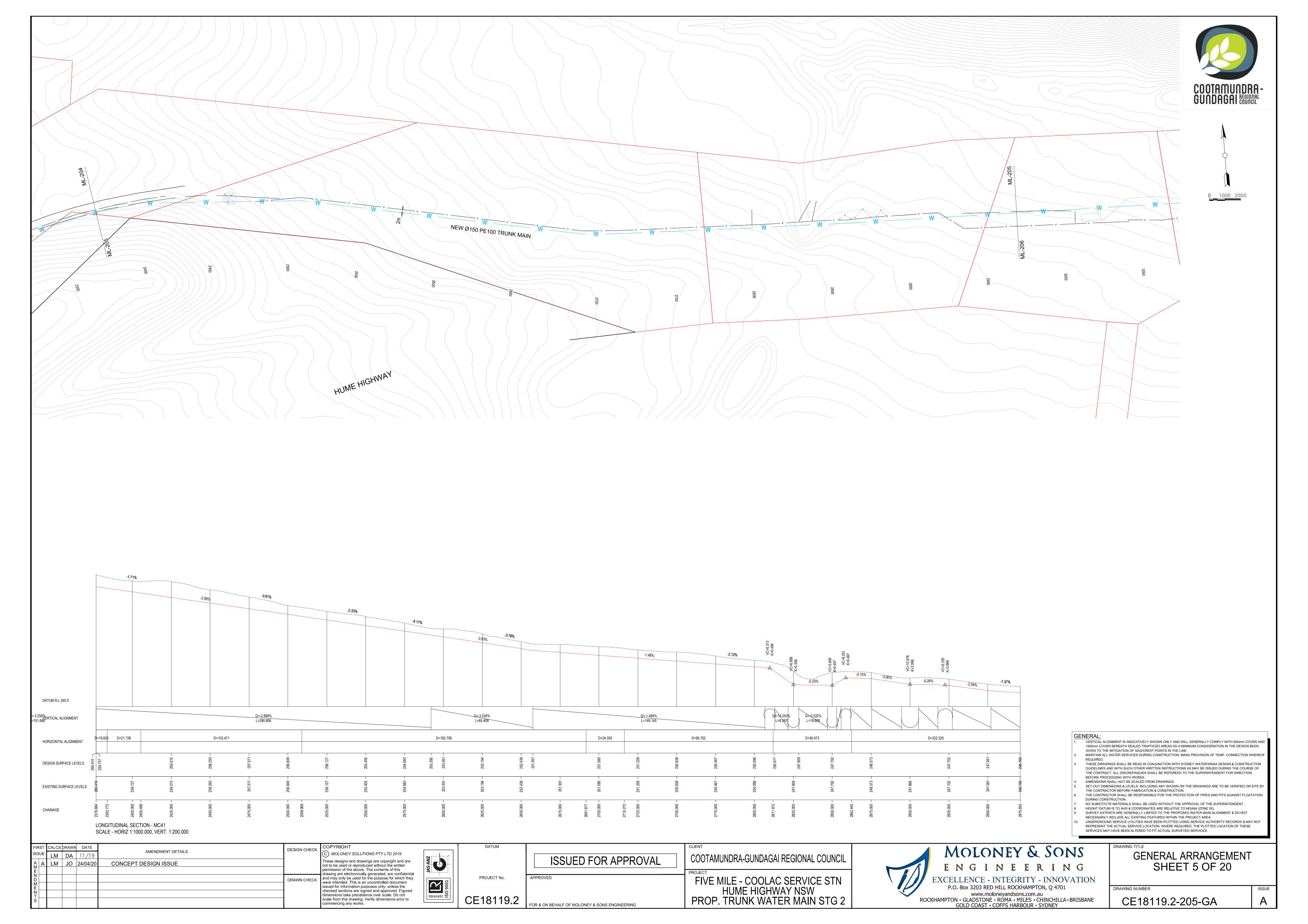
FIVE MILE - COOLAC SERVICE STN HUME HIGHWAY NSW PROP. TRUNK WATER MAIN STG 2

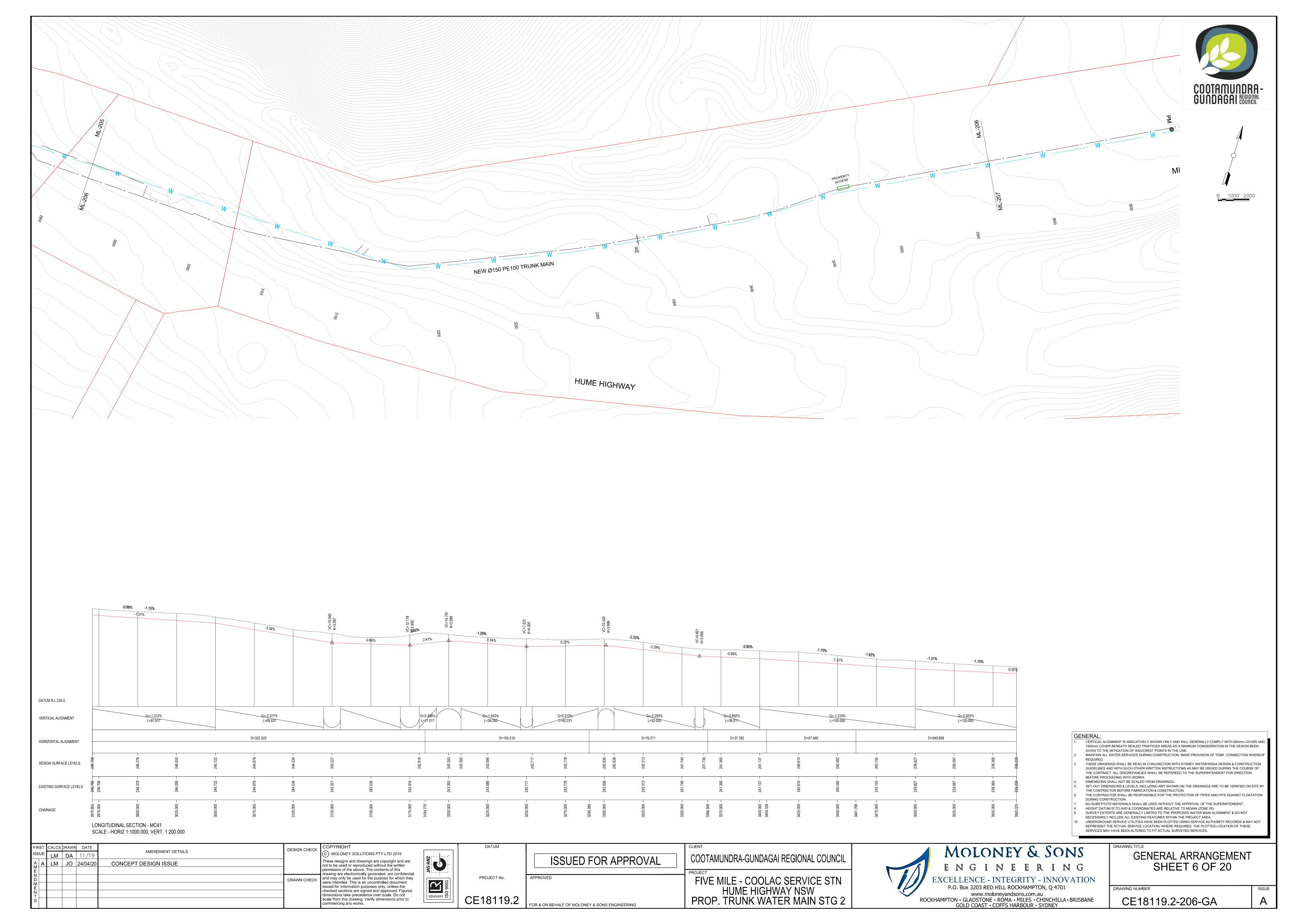


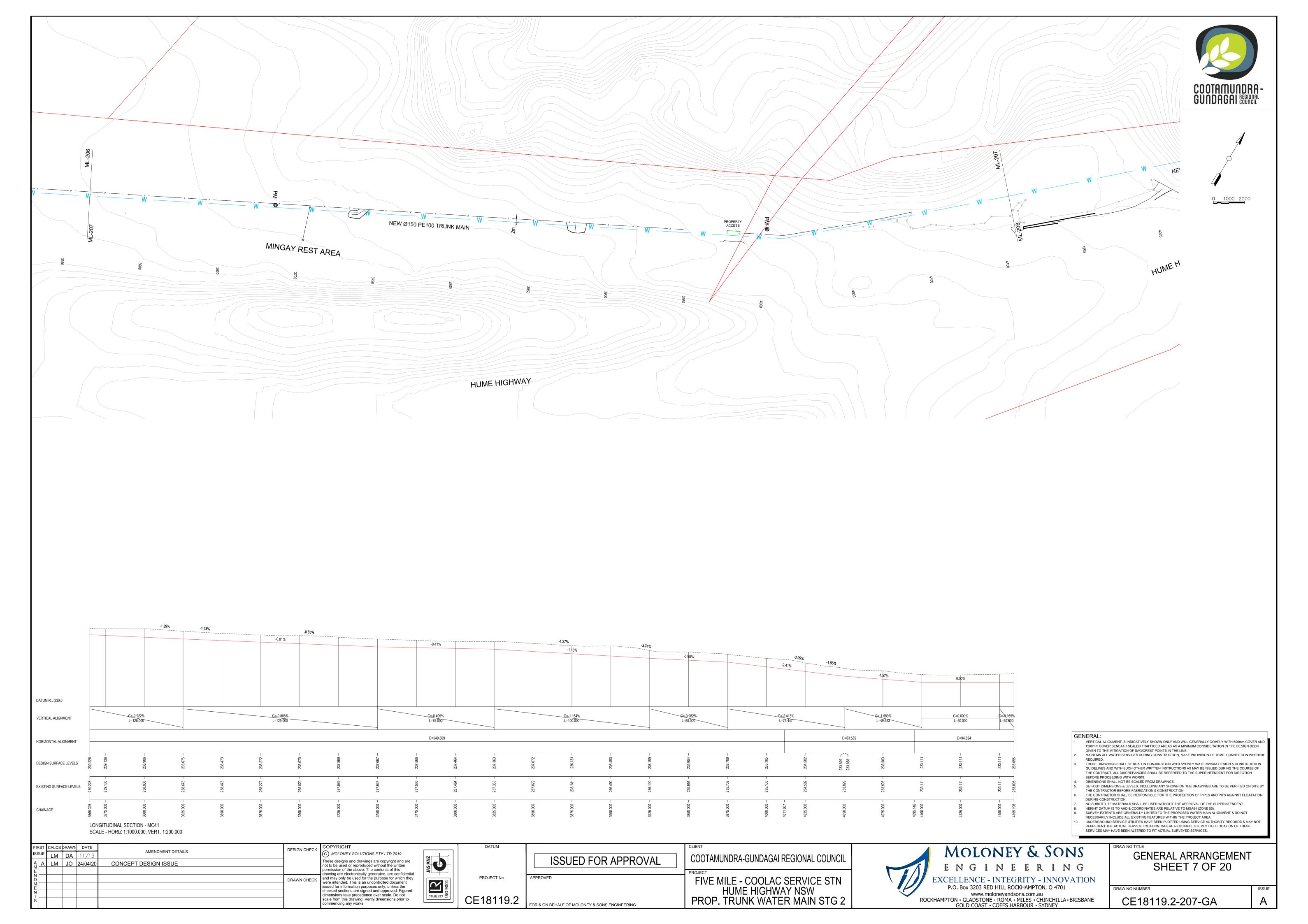


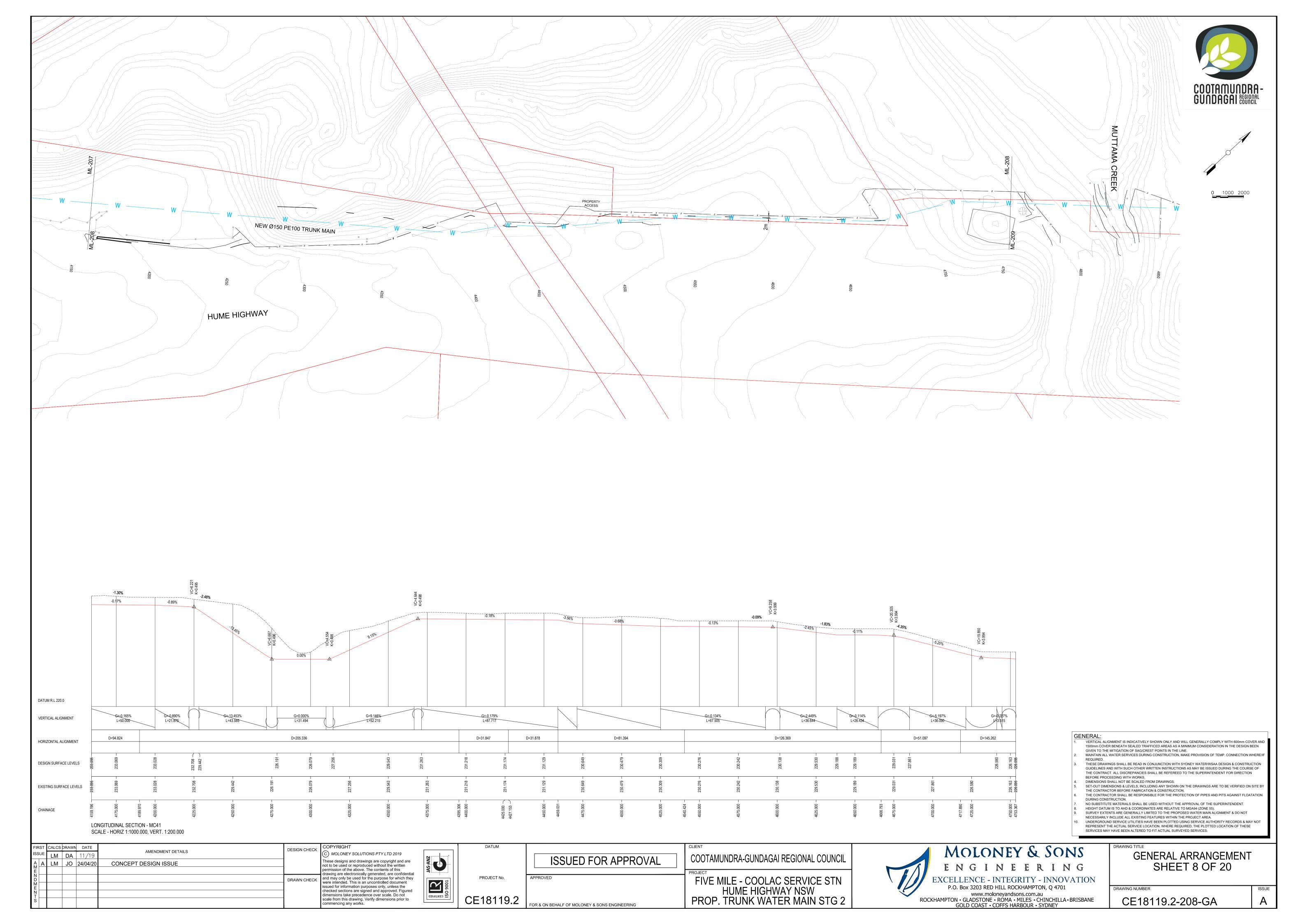


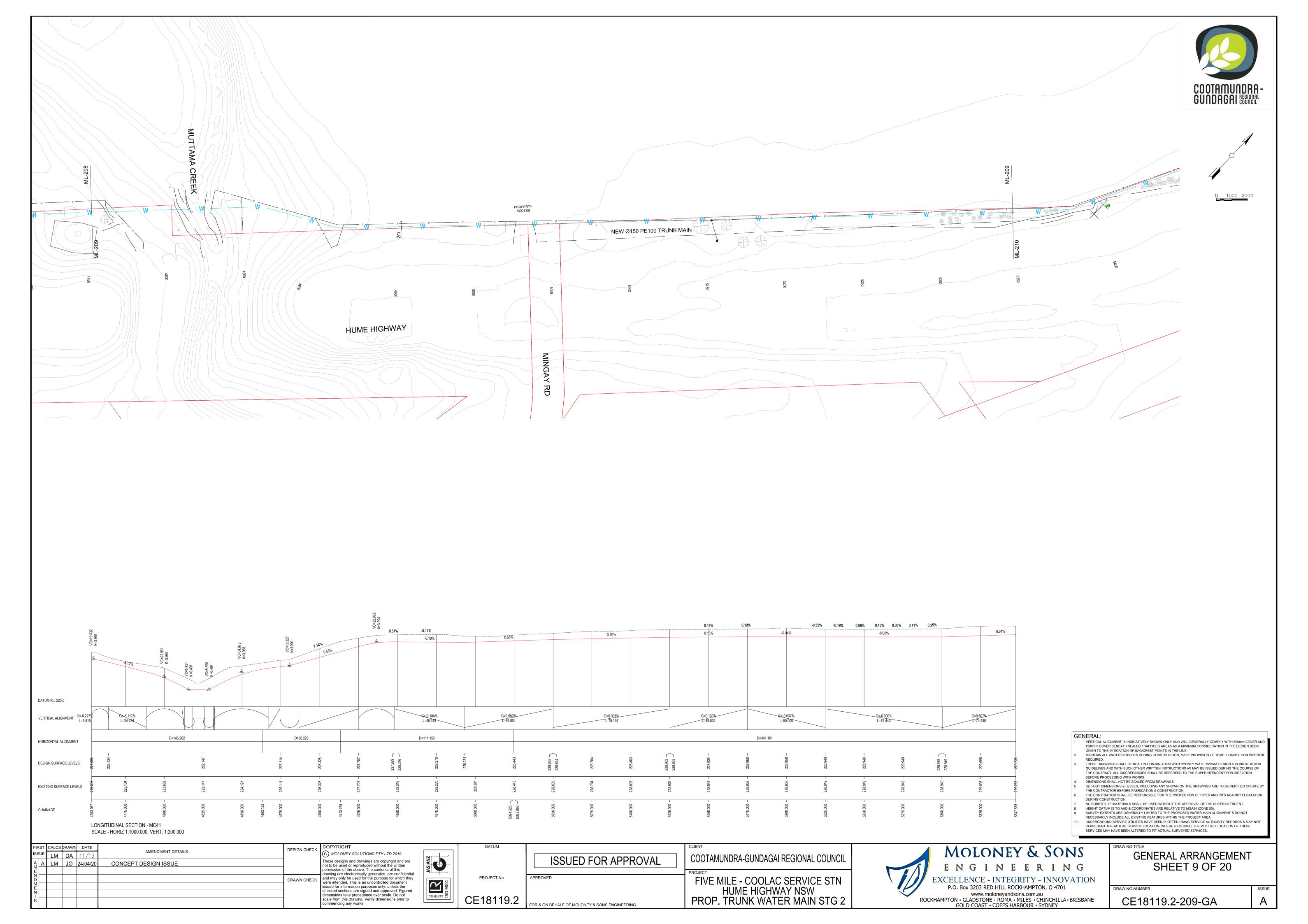


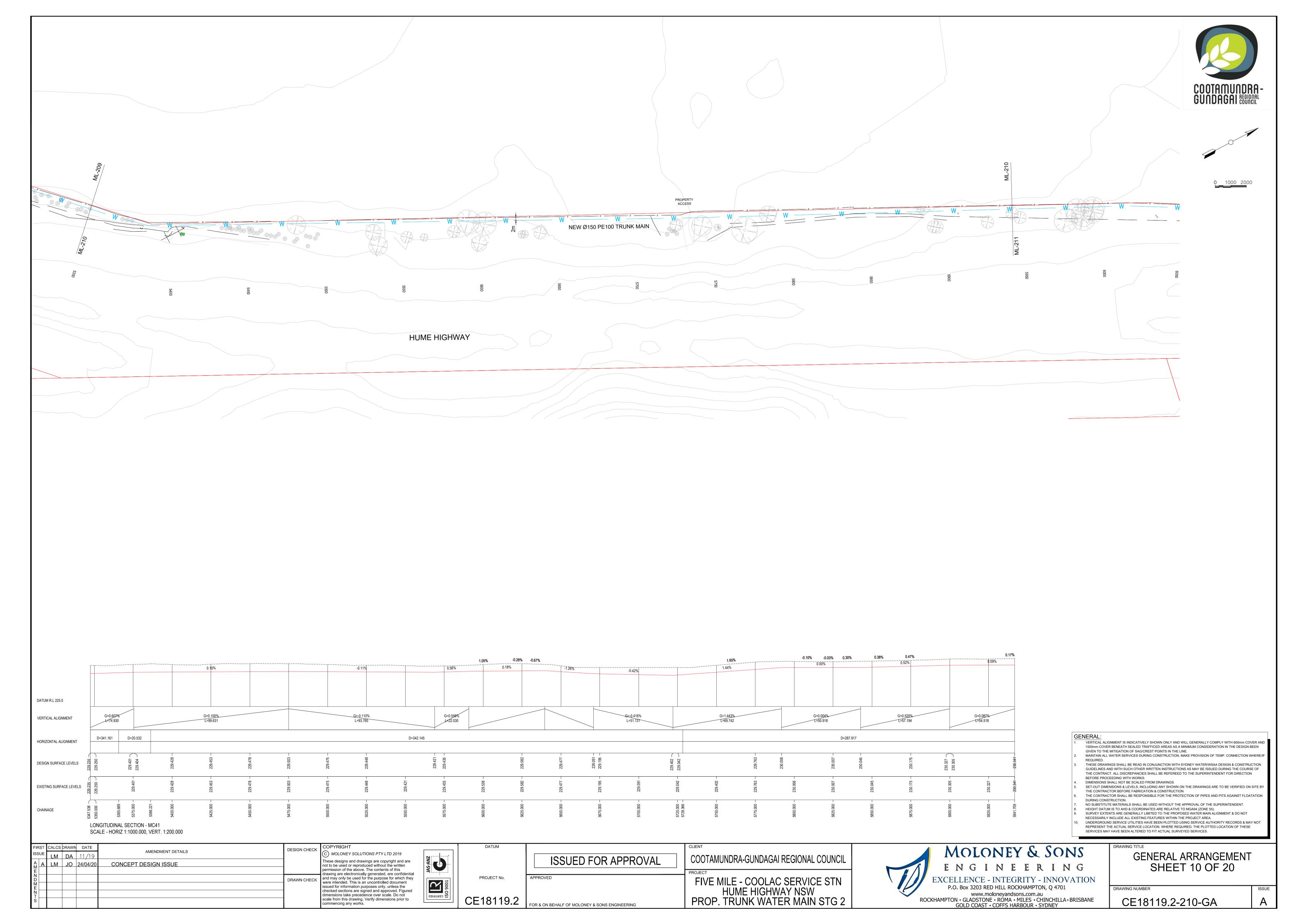


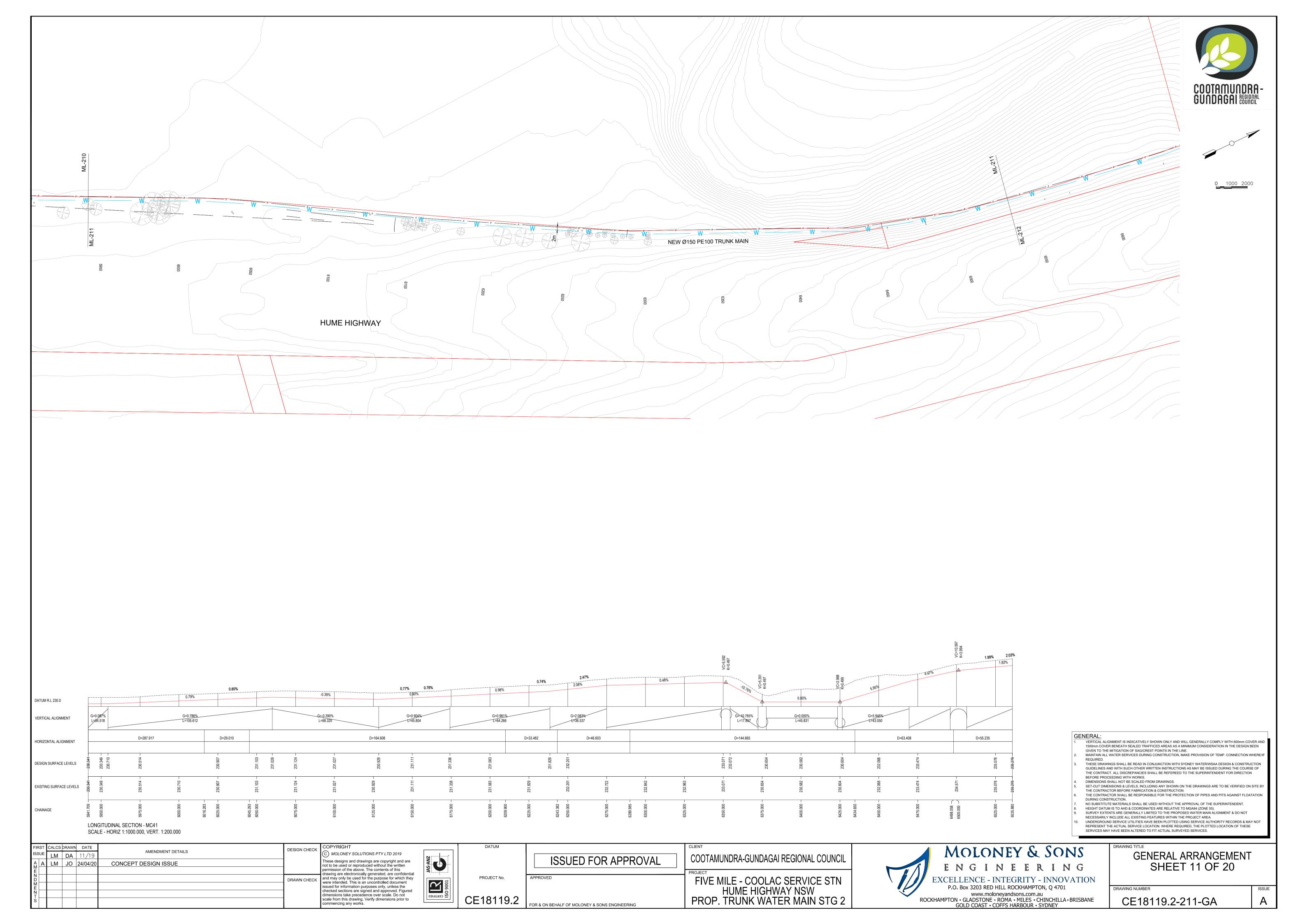


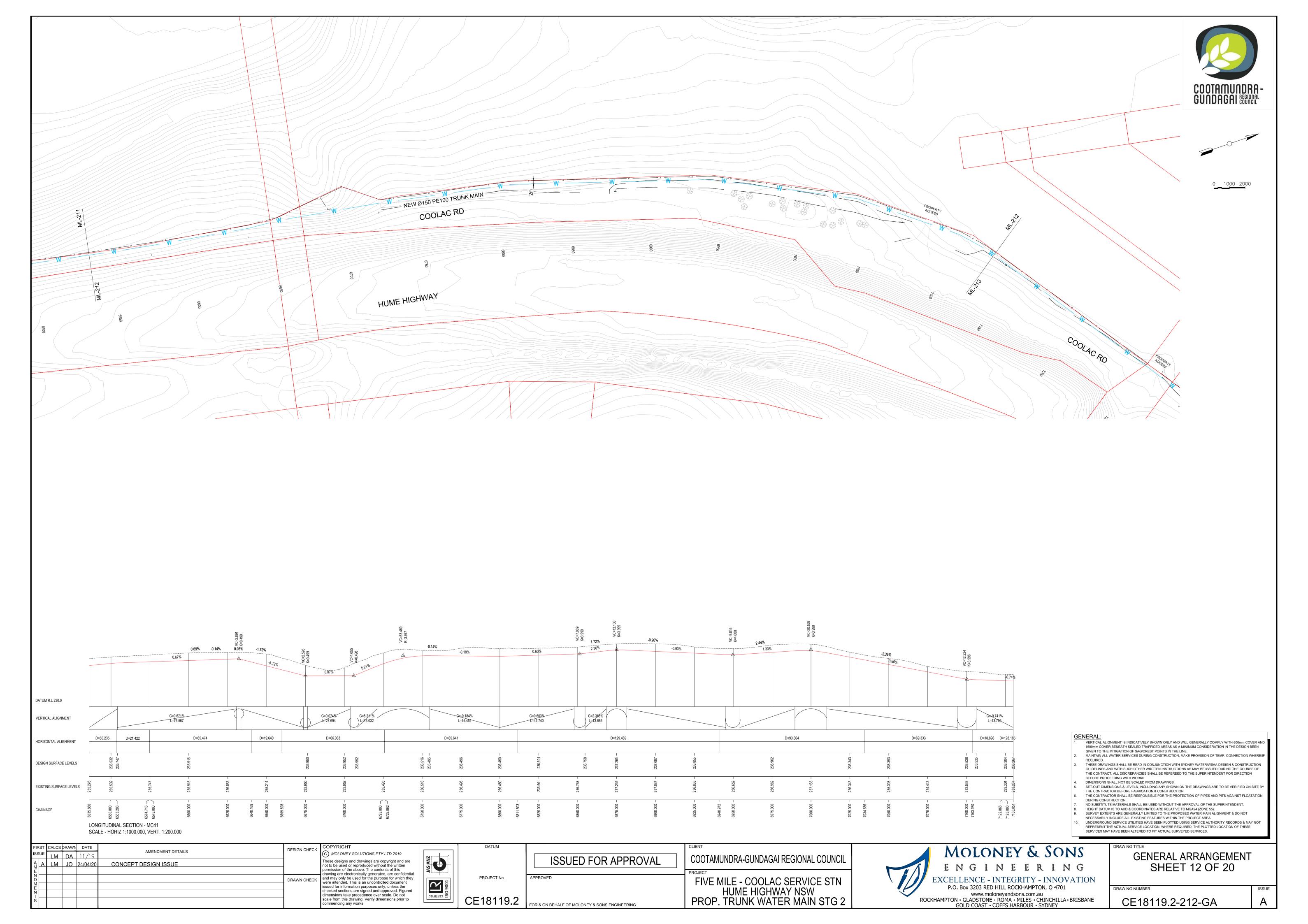


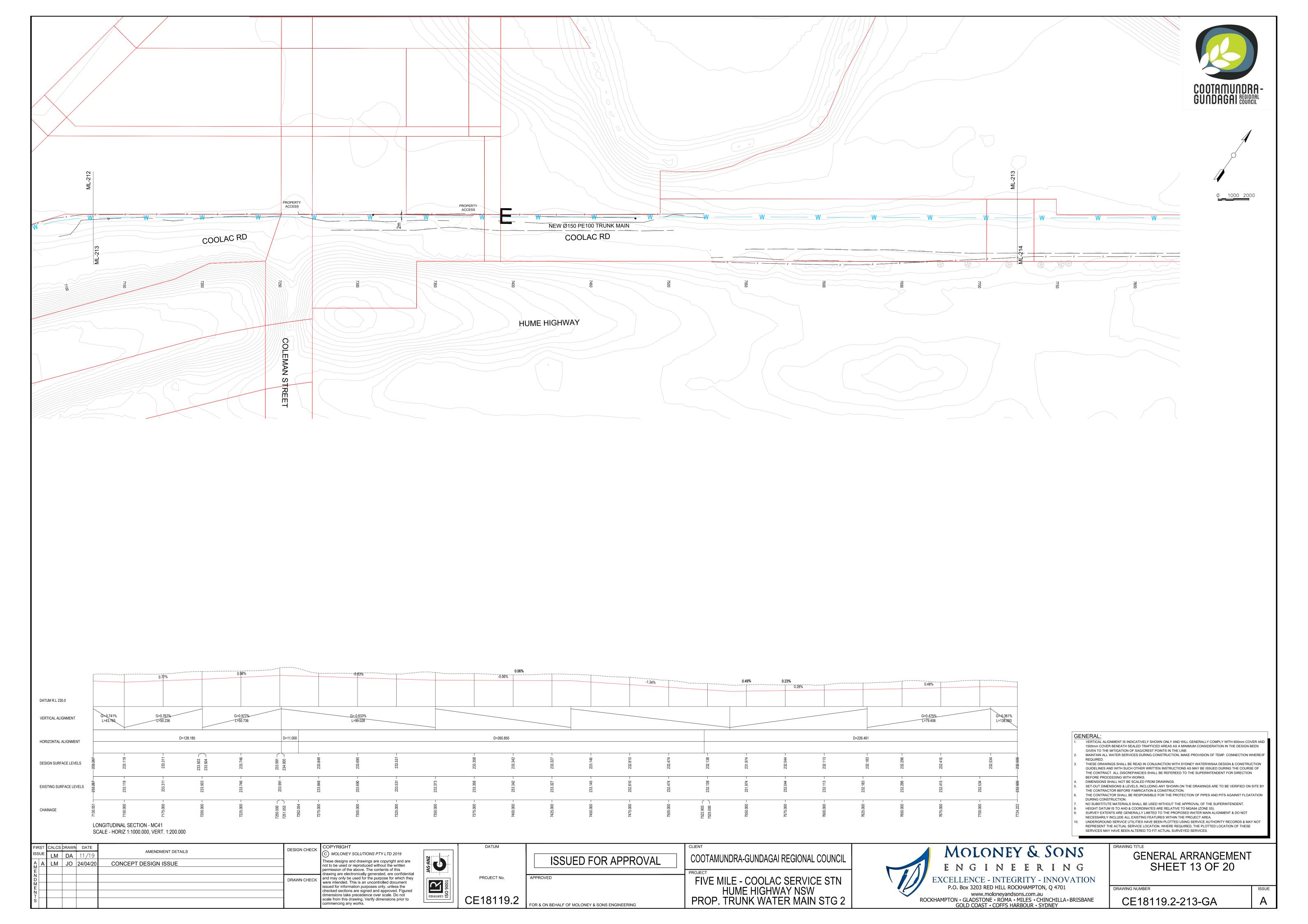


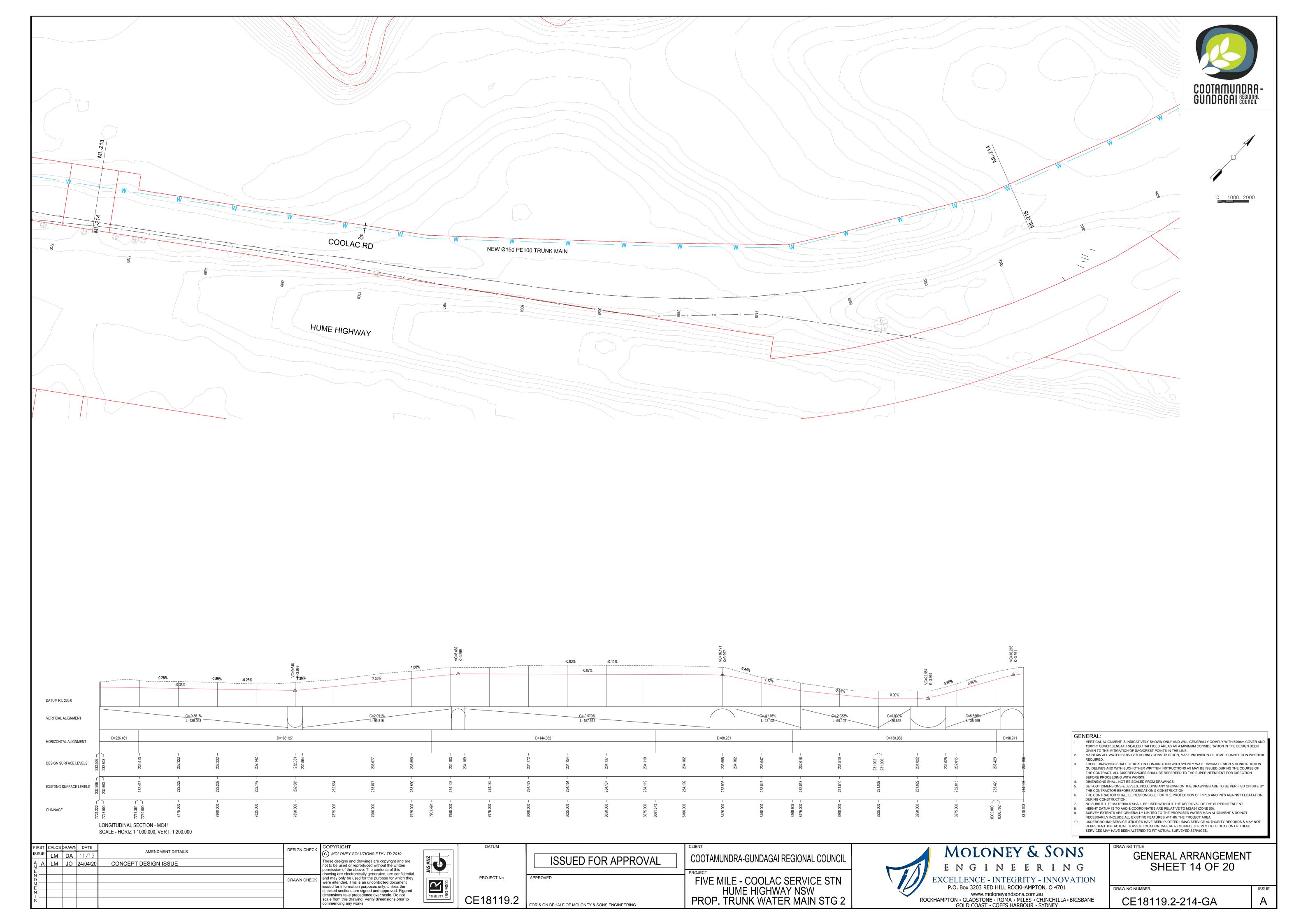


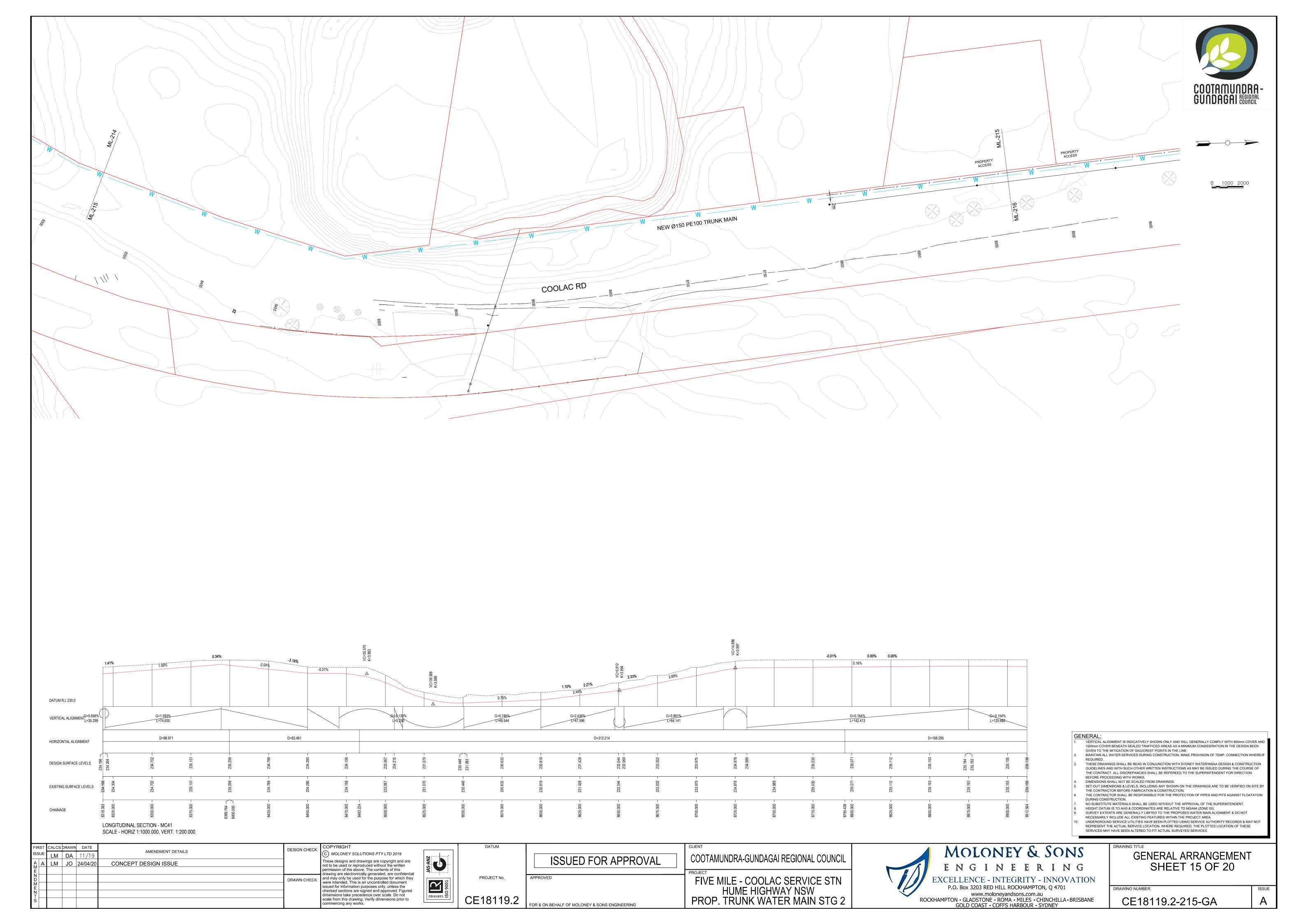


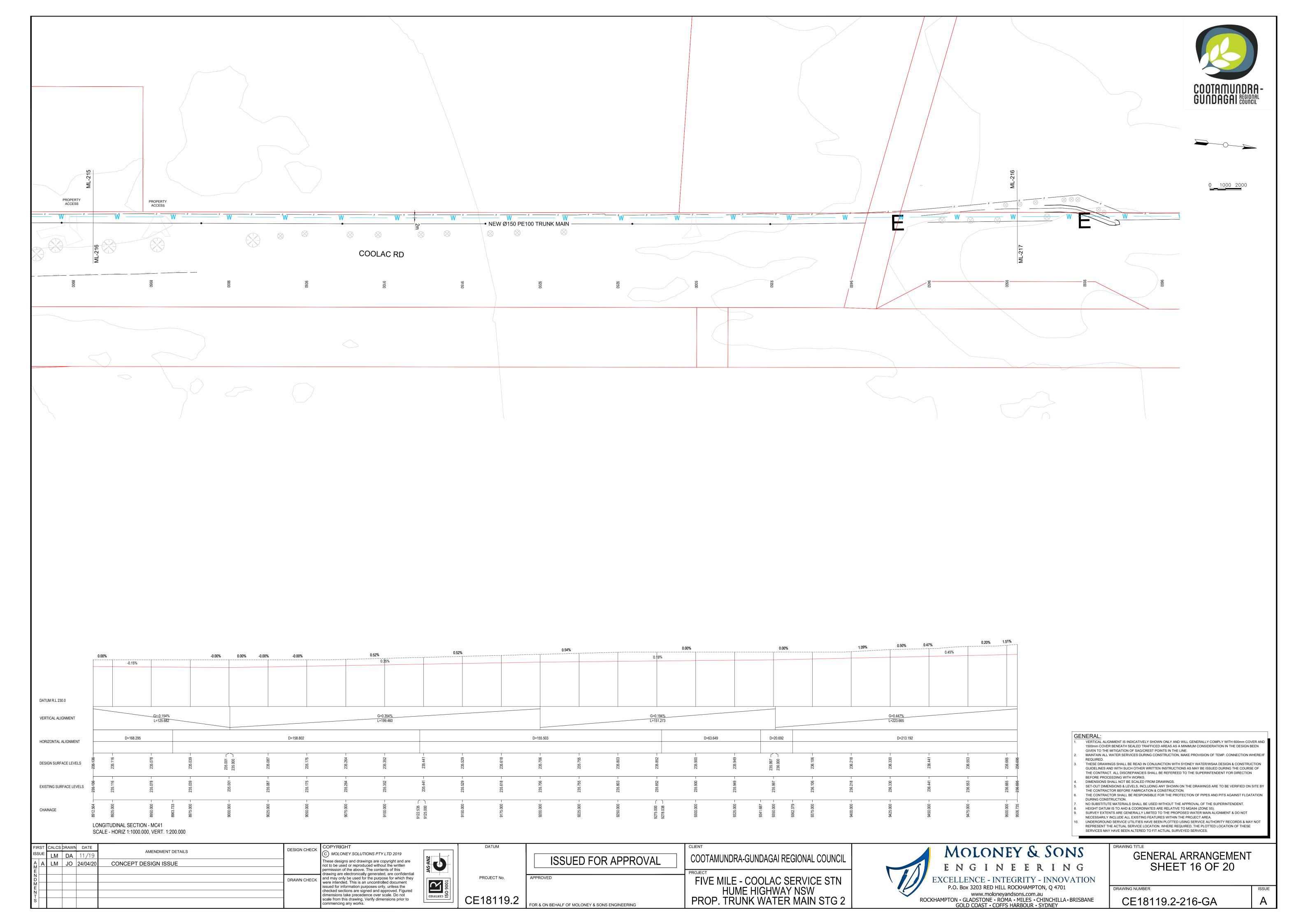


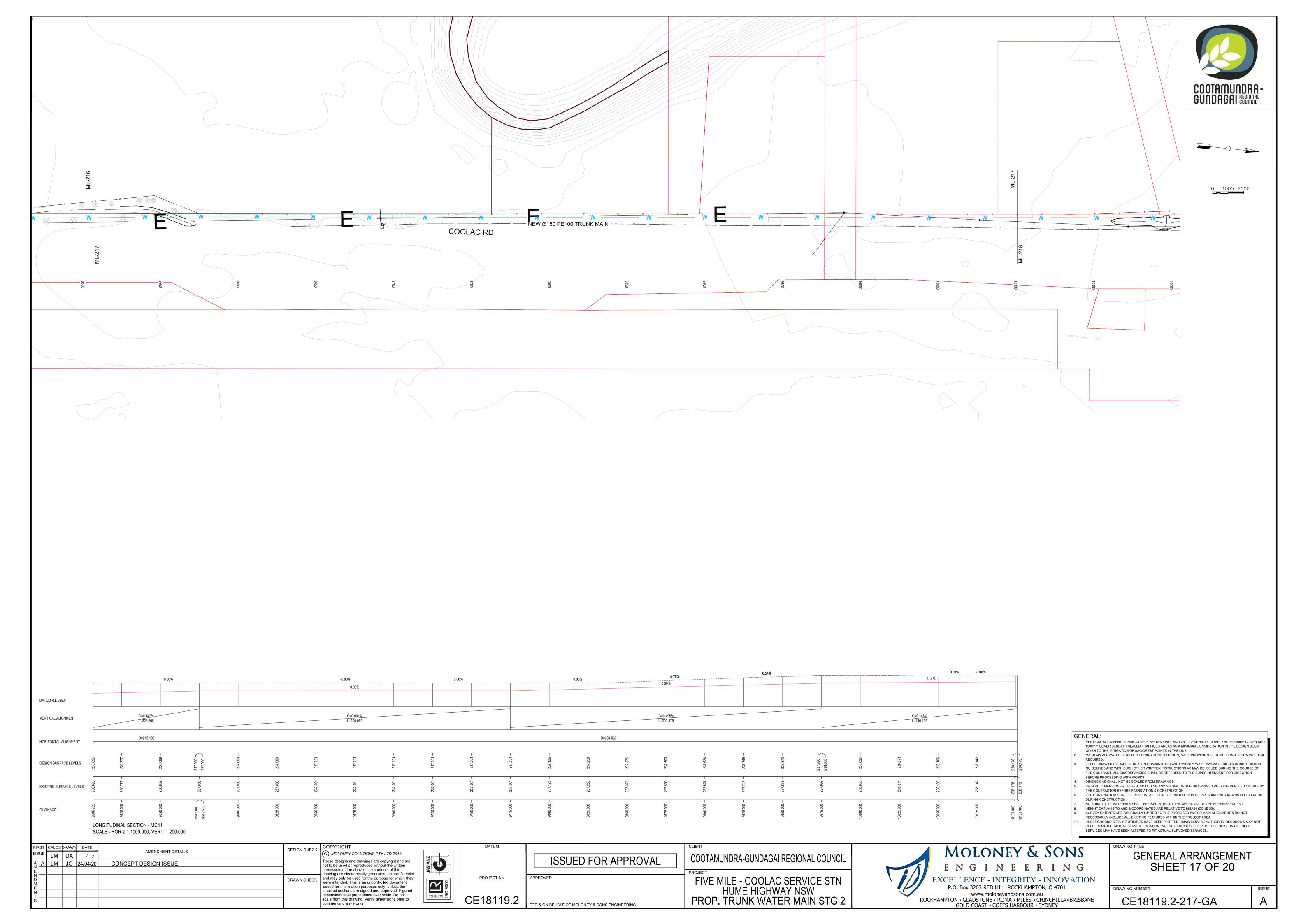


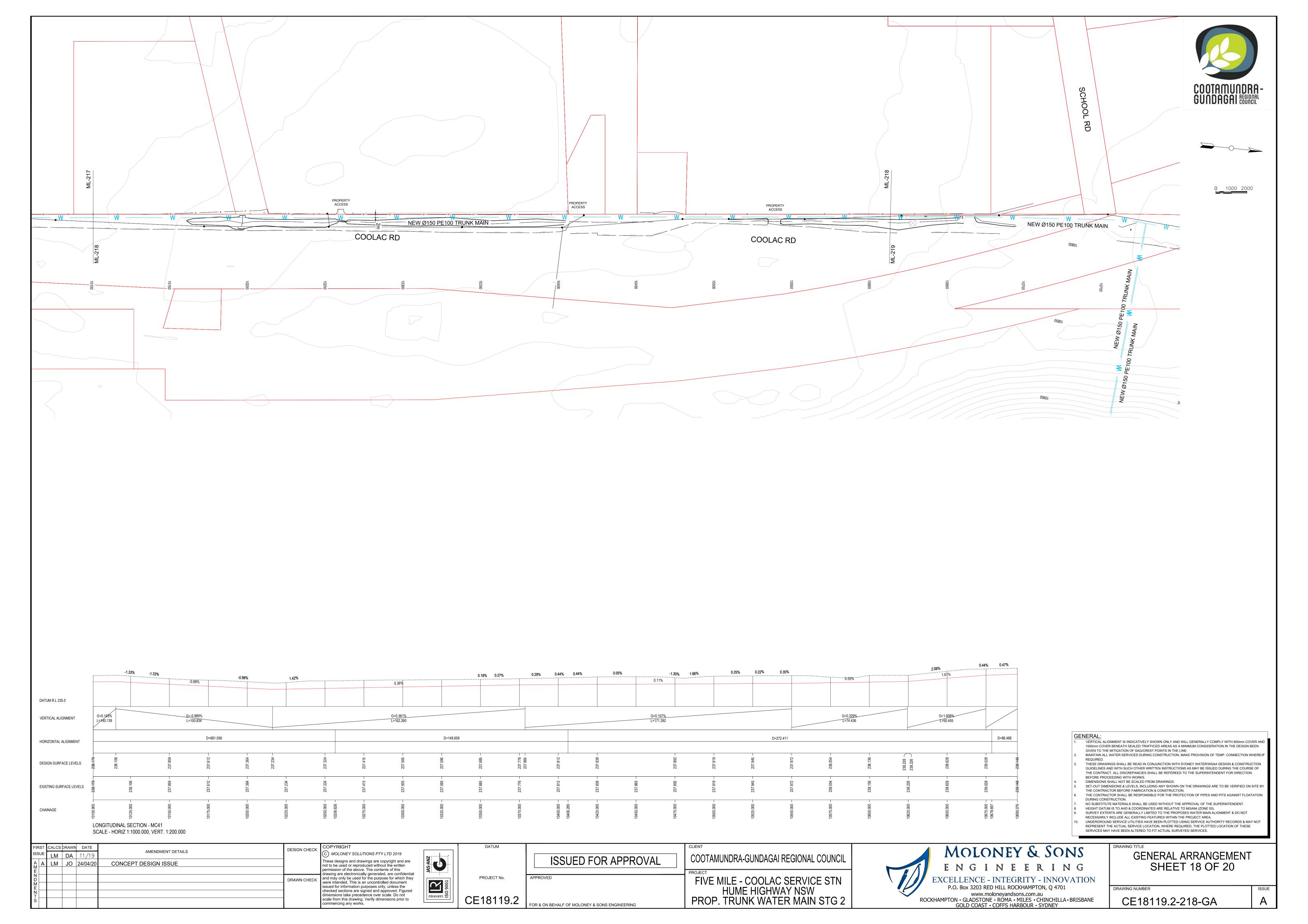


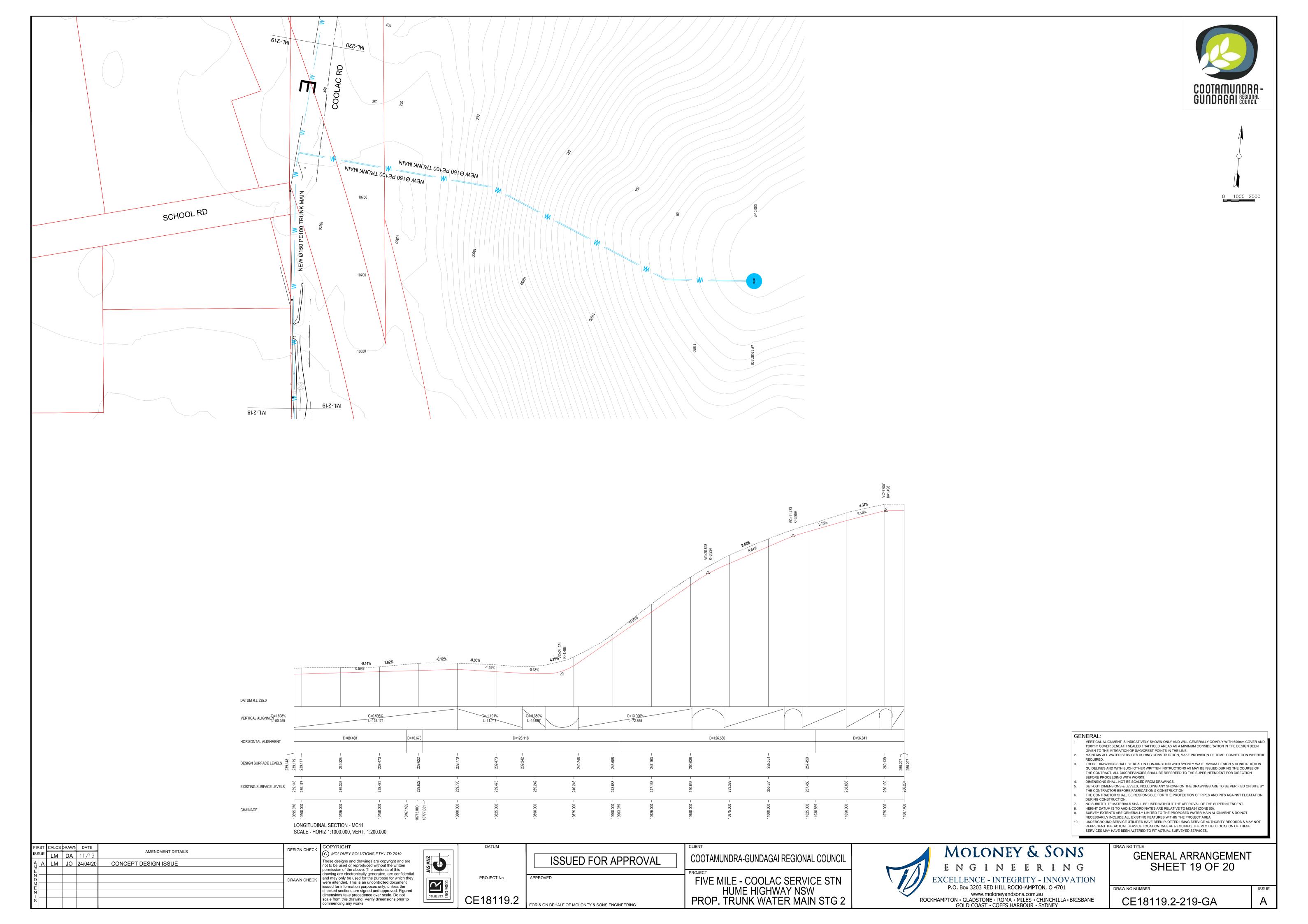


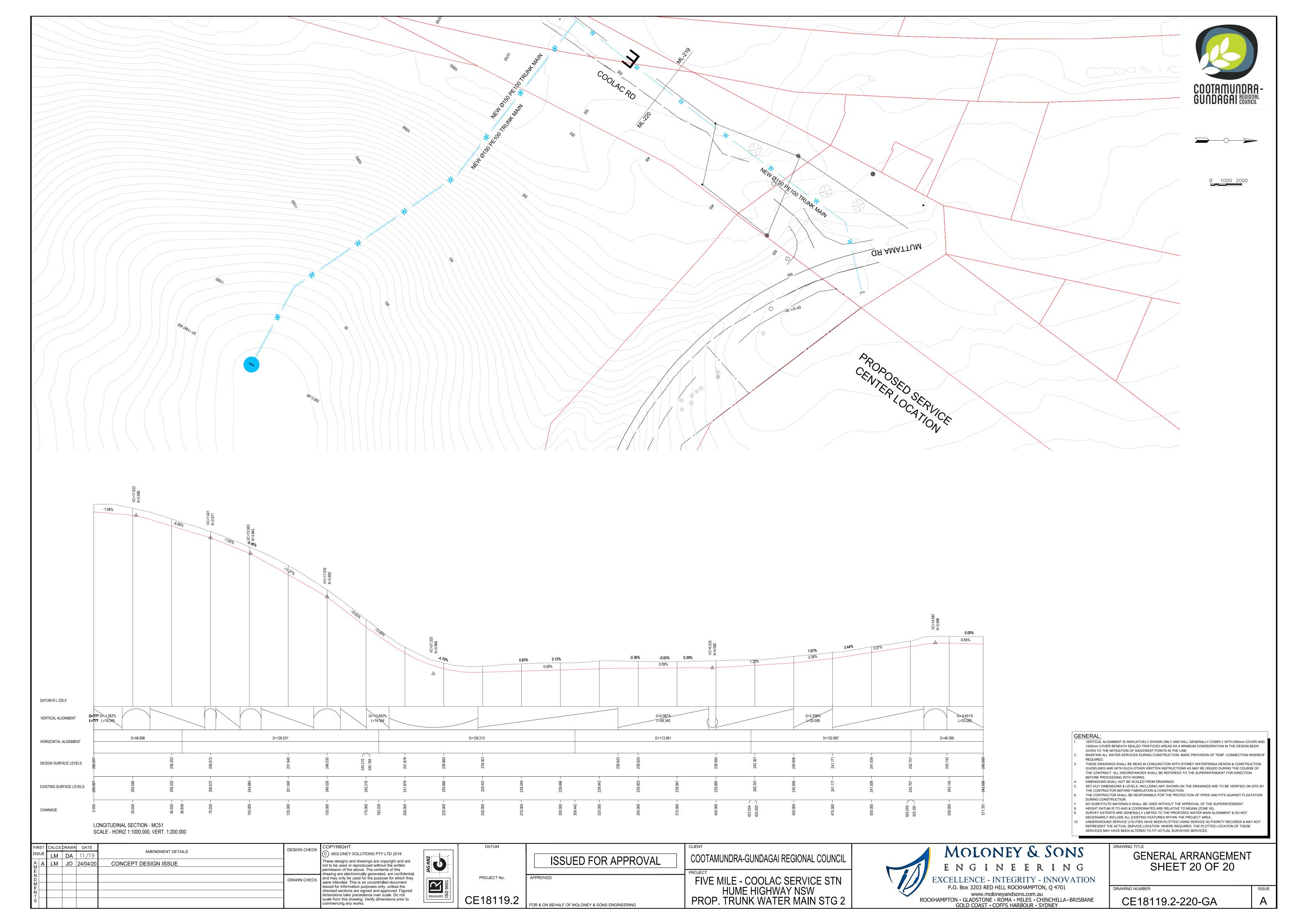














### APPENDIX C - INTERPRETED DIURNAL TABLES

			Corrected		Accumulative Consumption	Accumulative Consumption	Avg Accumulative	Avg Accumulative Consumption (m³)		Avg Accumulative Consumption (m <sup>3</sup> )
ne step Time		Residential		30 minute Consumption	(L)	(m³)	•	(24hr Pump Period)	Pump Rate m <sup>3</sup> /30min	(8hr Pump Period)
1	0:00			755.499			• • •	•		•
2	0:30			755.499			12858.750	12.859		
3	1:00		0.00244807	755.499						
4	1:30			755.499			25717.500	25.718		
5	2:00			819.797			32146.875			
6	2:30			819.797			38576.250	38.576		
7	3:00			819.797			45005.625			
8	3:30			884.094			51435.000	51.435		
9	4:00			945.177			57864.375			
10	4:30		0.003677313	1134.856				64.294		
11	5:00		0.006333729	1954.652		10.400	70723.125	70.723		
12	5:30		0.027793404	8577.322			77152.500	77.153		
13	6:00			11351.771			83581.875			
14	6:30			11792.211			90011.250	90.011		
15	7:00			11226.390						
16	7:30			10277.998			102870.000	102.870		
17	8:00			9332.821			109299.375			
18	8:30		0.025751609	7947.204			115728.750	115.729		
19	9:00		0.023293122	7188.491			122158.125			
20	9:30			6812.349			128587.500	128.588		
21	10:00			6558.372			135016.875			
22	10:30		0.021042982	6494.075			141446.250	141.446		
23	11:00			6494.075			147875.625			
24	11:30		0.021042982	6494.075			154305.000	154.305		
25	12:00			6368.694						
26	12:30		0.021042982	6494.075			167163.750	167.164		
27	13:00		0.021042982	6494.075			173593.125			
28	13:30		0.021042982	6494.075			180022.500	180.023		
29	14:00		0.021042982	6494.075						
30	14:30		0.021251328	6558.372			192881.250			
31	15:00		0.022272225	6873.431			199310.625			
32	15:30			7503.550			205740.000			
33	16:00		0.027178782	8387.644			212169.375			
34	16:30		0.030241473	9332.821			218598.750			
35	17:00		0.036168927	11162.093						
36	17:30		0.041690106	12865.983						
37	18:00		0.043523554	13431.804						
38	18:30		0.042908932	13242.125			244316.250			
39	19:00		0.041481759	12801.686						
40	19:30		0.038616997	11917.591			257175.000			
41	20:00			10847.034						
42	20:30			8953.464						
43	21:00		0.023095193	7127.408			276463.125			
44	21:30		0.017365669	5359.219			282892.500			
45	22:00		0.012667458	3909.304						
46	22:30		0.009396421	2899.829			295751.250			
47	23:00		0.004291935	1324.534						
48	23:30		0.002656416	819.797						
Total	25.50	95.994		308610.000		300.010	555510.000	300.010		